# STRENGTH CHARACTERISTICS OF CONCRETE CONTAINING POST CONSUMER METALIZED PLASTIC WASTE

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## Abstract

This paper represents results of strength tests conducted on concrete containing flakes of waste metalized plastics. The feasibility of post consumer plastic waste (PCMPW) was determined on the bases of tests results of compressive strength and split tensile strength. Ever increasing generation of PCMPW has raised littering issues and has become one of the major sources for landfills. The objective of the test program was to check the feasibility of PCMPW as concrete constituent and possible way of mitigating the environmental impacts occurring from insufficient and unsafe management of PCMPW. The strength tests were performed with three sizes of PCMPW in flaks and fiber form. PCMPW was added in the mixes with 0%, 0.5%, 1%, 1.5% and 2% by volume of concrete mix. Test results reveled that strengths reduced with increased PCMPW flakes contents. However, up to 1% of addition of flakes the strength reduction was negligible for all flak sizes. Moreover, the PCMPW fiber with 1mmx17mm size improved the split tensile strength. Experimental results of compressive -split tensile strength were correlated by a standard analytical model and exhibited good agreement with the experimental results. Such concrete could be utilized in pavements, substructural components and in non-structural members.

Key Words: Post consumer metalized plastic waste, Concrete, Compressive strength, Split tensile strength, Slump,

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Compaction factor.

## **1. INTRODUCTION**

Use of plastic wastes in concrete has provided an innovative way of safe disposal alternative. Experimental work could be found in literature wherein the feasibility and effects of addition of waste plastics have been carried out by many researchers [3,5].

Plastic waste resulting from various post consumer activities has been a major source of pollution and specifically major littering cause [7]. Food packaging industries have been using metalized plastic in most of the products. Such wastes are found difficult to treat for recycling and re-use. Even in present time, most of such food packaging plastics are disposed as a part of landfill. Landfill on the other hand, raises environmental and socio-economical issues [7].

Researchers therefore have used post consumer plastic wastes in concretes. It has been found that use of PET bottles have been utilized in most of the works. In many of the cases the addition of PET bottles in various forms like flakes and aggregates provided feasibility of plastic waste utilization [6,8,9]. However, negligible work has been carried out on use of metalized plastics - used for food packaging- as concrete constituent.

The present work focuses the feasibility of addition of metalized plastics used in different food packaging. Metalized post consumer plastic films were shredded into flakes of different sizes and added in the concrete mix in different proportions. Workability and basic strength properties namely compressive and split tensile strength of concrete were investigated. Analytical relationship between strengths was obtained. The results demonstrated that in all test conditions the concrete containing metalized waste plastic maintained strength properties up to 70% at 1% dosage of plastic flakes. Such novel concrete could preferably used in secondary structural elements and pavements. The experimental program showed potential towards the use of metalized plastic in cement concrete as an alternative way of mitigating the disposal challenges.

## 2. RESEARCH SIGNIFICANCE

PET bottle wastes have been utilized by many researchers towards making the concrete sustainable. Plastic wastes resulting from the post consumer activities, especially from food packaging contributes to the littering and landfill issues due to the fact that such plastics could not be effectively utilized for recycling or re-use. Therefore the principal objective of this study was to check the feasibility of use of waste metalized plastic as concrete constituent. The results of tests on concrete containing different plastic waste contents and sizes were utilized towards predicting the effective use of waste plastic as concrete constituents.

## **3. EXPERIMENTAL PROGRAM**

## 3.1 Materials, mixes and casting

Ordinary Portland cement (OPC) of 53- grade normally used in conventional concrete was used to produce the concrete mix. Locally available aggregates of 20mm and 10mm sizes with ordinary river sand were used. Metalized plastic film was received from the local manufacturer. The film was shredded in three different sizes in flake form. The general properties of metalized plastic films are tabulated in Table 1. Three mixes were prepared with different sizes and proportions of waste plastic flakes as shown in table 2. The water to cement ratio was maintained at 0.45 for all mixes. Plastic films were shredded into three different sizes in flakes form as shown in figure 1. The average flake size was 1mm, 5mm and 17mm respectively. The mix proportions of concrete constituents are shown in table 3.



Fig.-1 Shredded PCMPW flakes

Table -1: Properties of PCMPW films			
Property	Value	Unit	
Resin category	Polythene		
Plastic type	LDPE		
Density range	0.94-1.4	gm/cm <sup>3</sup>	
Thickness	0.05	mm	
Tensile strength	1800-1900	kg/cm <sup>3</sup>	
Elongation	90 -110	%	

Table -2: Mix proportions			
Mix	Flake size mm	Flake contents %	
Mix 1	1	0, 0.5, 1, 1.5, 2	
Mix 2	5	0, 0.5, 1, 1.5, 2	
Mix 3	17	0, 0.5, 1, 1.5, 2	

#### Table -3: Concrete mix proportions

Mix m <sup>3</sup>	Cement kg	Water L	Sand kg	Agg. 20mm kg	Agg. 10mm kg
1	383	202.5	644.4	744	496

Total 30 specimens were casted with 150mmx150mmx150mm standard cube molds and 150mm diameter with 300mm height of cylinder molds. Utmost care was taken during the entire process of mixing and casting of the specimens. The specimens were casted for 28 days in potable water and tested in the UTM as per the standard guide lines.

### 3.2 Testing

The specimens were tested for obtaining compressive strength and split tensile strength. The compression testing machine of 2000KN capacity was used. Figure 3 shows specimens under testing conditions. Fresh concrete mixes were utilized to determine the slump and compaction factor. Fresh behavior of the mixes containing plastic flakes was studied by measuring the slump as in the form of workability and consistency of mix was studied by obtaining the compaction factor of the mixes.



Fig. – 2 Compressive strength apparatus



**Fig.** – **3** Split tensile strength apparatus

## 4. RESUTLS AND DISCUSSION

#### 4.1 Fresh properties of concrete mixes

Table 4 summarizes the slump and compaction factor test results. The effects of addition of plastic flakes were studied from the fresh behavior of mixes.

It was observed that the slump and compaction factor values reduced with the addition of the plastic flakes. However the reduction scenario found to be different for slump and compaction factor with the variation in the size and content of the plastic flakes. Addition of flakes in varying sizes reduced the slump up to 30% at full dosage of 2%. The slump indicates the workability of the concrete mix. Knowing the fact that workability largely depends on the water to cement ratio of a mix, the cement paste in the concrete found responsible for the workable concrete mix. Inclusion of flakes interrupted the sufficient paste formation in the mix as observed and resulted in the stiff and reduced workability of the concrete mixes.

Table 4: Slump and	compaction factor results
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Batch	Percentage of plastic waste	Slump	Compaction
no.	flakes	(mm)	factor
1	0 % plastic flakes	86	0.91
2	0.5% plastic flakes	75	0.88
3	1 % plastic flakes 1mm	58	0.85
4	1.5% plastic flakes	48	0.82
5	2 % plastic flakes 1mm	35	0.73
6	0.5 plastic flakes 5mm	70	0.85
7	1 % plastic flakes 5mm	45	0.82
8	1.5 % plastic flakes	36	0.76
9	2 % plastic flakes 5mm	25	0.69
10	0.5% plastic fibre	68	0.89
11	1 % plastic fibre 17mm	56	0.86
12	1.5 % plastic fibre	43	0.77
13	2 % plastic fibre 17mm	32	0.71

Compaction factor measures the degree of compaction of a mix. A workable concrete generally provides compaction up to the satisfactory level. Plastic flakes have been nonreactive to water and any other constituents in the concrete mix. Possessing very less thickness of 0.05mm, the presence of the flakes did not affect the compaction characteristics as in the case of slump. The concrete possessing the flakes of varying sizes found compactable up to the acceptable degree. This could be found from the results of tests performed on various mixes as shown in table 4.

It was observed that with the increased size of waste plastic flakes the slump was reduced rapidly for all contents of flakes. There was a gradual reduction noticed with the increased size of flakes in the slump values with the increased dosage of flakes. Results of slump revealed that with full dosage of waste plastic flakes of 2% by volume of mix provided acceptable range of workability and could be utilized for pavements and hand compacted concrete used in foundations. Similarly for compaction factor found to be in between 0.69 to 0.88 with varying flakes content provided acceptable degree of compaction showing the maximum reduction up to 24% at full flakes dosage.

#### 4.2 Strength properties hardened concrete

Figure 4 shows average value of compressive strength and figure 5 shows split tensile strength of concrete containing varying proportions and sizes of waste plastic flakes. Results showed reduction in compressive strength with the increased flakes content with the varying sizes. Addition of flakes reduced the strength up to 30 % for full dosage of flakes as 2% by volume.

## 4.2.1 Compressive strength

Compressive strength was affected by addition of plastic waste flakes and showed reduction with increased flakes contents. Addition of 0.5% of flakes showed reduction up to 5% for all three sizes of flakes. For 1% of flakes contents the strength reduced further up to 8%. The maximum reduction at the dosage of 1% of flakes was noticed with 5mm size. On further increasing the contents from 1% to 1.5% of flakes the strength reduced up to 39% in case of 5mm size of the flakes. However the reduction in case of 1mm and 17mm sizes were 18% and 33% respectively. At a full dosage of 2% of flakes, the strength reduced up to 44% in case of 5mm flakes. The reduction of strength in case of 1mm and 17mm size flakes was found to be 36% and 42% respectively. It was noticed that larger size of the flakes reduced the strength effectively. The aggregates covered with cement paste results in a better compressive strength. Inter molecular bonding of aggregates and cement paste results in strong adhesion and provides better strength. At the phase around the aggregate where coating of cement paste should be maximum get disrupted by the larger size of flakes. Additional the effective inter molecular friction which is essential for constituent adhesion also reduced due to the larger size of the flakes. In case of small size of flakes

like 1mm did not affect the overall molecular friction and showed less strength reduction.

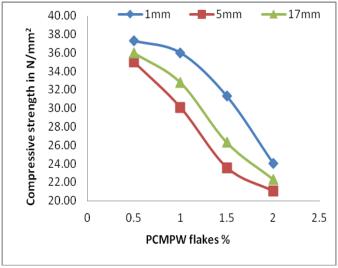


Fig.-4 Effects of PCMPW flakes on compressive strength

#### 4.2.2 Split tensile strength

The split tensile strength reduced with the increased flake content and with varying size of flakes. However it was interesting to note that the split tensile strength increased for the flakes dosage up to 1% in case of 17mm size flakes. This was an important finding form the study on the concrete response containing waste plastic flakes. The strength reduced up to 43% at full dosage of flakes for 1mm size flakes at 2% addition and up to 47% in case of 5mm size flakes at 2% of addition. The strength was increased up to 18% at 1% addition of 17mm size flakes. The cement paste contains voids and internal micro cracks responsible for weak tensile strength of concrete. Addition of long size flakes in the concrete helped to improvise the tensile capacity of the paste and contributed to the better grip of paste on the constituents which resulted in the improved split tensile strength.

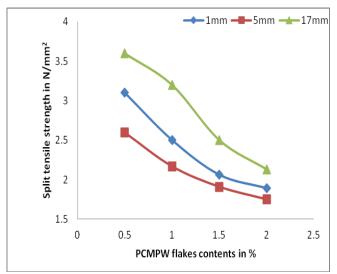


Fig- 5 Effects of PCMPW fakes on split tensile strength

## 4.2.3 Relationship between compressive and tensile

#### strength

The relation between tensile strength and compressive strength of PCC can be expressed in form of power equation as  $f_t = k * (f_c)^n$ , where k and n are constants.

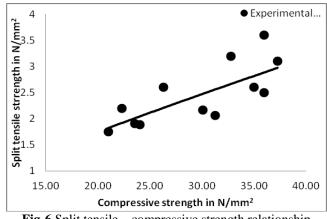


Fig-6 Split tensile - compressive strength relationship

As shown in figure 6, the experimental values of compressive-split tensile strength exhibited a power relationship as suggested by a standard relationship format. The analytical expression between split tensile strength  $f_t$  and compressive strength  $f_c$  was found as following,

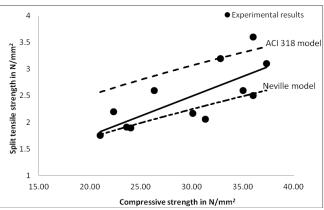
$$f_t = 0.14 * (f_c)^{0.85}$$
 ... (A)

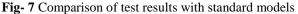
Different researchers have proposed different values of 'k' and 'n' which is mainly attributed to the difference in loading rates and constituents' proportions. Most commonly adopted relationships between tensile and compressive strength are as follows,

Neville equation [1]:  $f_t = 0.23 * (f_c)^{0.67}$  (B)

ACI 318-08 code [2] 
$$f_t = 0.56 * (f'_c)^{0.5}$$
 (C)

By employing the above standard equations on the Experimental results the graphs were developed as shown in figure 7.





From the figure 7, it could be observed that none of the existing analytical relationship was found suitable to predict the compressive-split tensile relationship. Neville equation (B) provided less reduction in the split tensile strength based on the compressive strength value compared to the ACI 318 (C).

The crack phenomenon in the concrete under compression has remained dependant on more than a single factor or variable. It has been found generally that the tensile strength of concrete depends on the compressive strength of the mix. However, the tensile resistance could be improved by improvising the ductility of the hydrated cement paste as observed from the work done on fibre reinforced concrete. Addition of plastic waste flakes contributed to the improved tensile resistance of the cement paste and provided better split tensile strength to the concrete.

The analytical model shown in (A) exhibited by the experimental values, suggested that compared to the standard relationship models the experimental values were found higher in case of the power term of the equation. Similarly the scale factor as prefix in the equation found to be lower than that of the standard models. The analytical relationship received from the experimental data, showed that the concrete containing PCMPW flakes showed better resistance to cracking in concrete compared to the conventional concrete. Moreover, the standard relationship counted the split tensile strength of concrete very low which did not agreed with the experimental results of concrete containing PCMPW flakes as the resistance to cracking was improved for concrete containing PCMPW flakes.

#### **5. CONCLUSIONS**

Addition of waste plastic flakes in fibre form of 17mm length significantly improved the split tensile strength up to the contents of 1% by volume. Flakes of size 5mm and 1mm reduced the compressive strength significantly up to 30% at full dosage of 2% and found not suitable for application. 1% of waste plastic flakes addition provided acceptable response of the concrete mixes for all test conditions in all sizes of flakes. The compressive – split tensile strength relationship obtained from the experimental results also supported the improvement of tensile resistance of concrete containing PCMPW flakes. Addition of metalized plastic waste found feasible in concrete as constituent and showed research potential towards sustainable construction material development.

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