

SEISMIC COMPARATIVE STUDY ON TMD STRUCTURE: AN OVERVIEW

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Abstract

In larger context of civil engineering use of mechanical vibration control device is very often in present days, so this paper will provide the successful implementation specification of TMD to reduce the vibration induced due earthquake ground motion, with computer stimulated model & dynamic responses are obtained for seismic loading with & without TMD which drastically reduces seismic response such as lateral displacement, story drift, base shear, overturning moment. Even actual behavior study on structure has been carried by inducing ground motion to the structure by considering time history data of El-Centro & recent earthquake (2014) which happened in India.

Keywords: TMD, Time History, Base Shear, Story Drift, El-Centro etc...

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1. INTRODUCTION

It is interesting to note that the use of active and passive devices is practice since from 1999 in India with first successful installation in killari village after suffering great loss of life and money due to earthquake caused in the year 1993.

The base isolator is very firstly introduced in killari village for the single story building. Coincidentally building is located very near to the location of Killari Earthquake epicenter. Again second application is done to hospital building by learning lesson after great loses in life from 2001 Bhuj earthquake in Gujarat. All though there is no section related to analysis, design implementation provisions are not available in IS codes except some special draft prepared by GSDMA. So this paper made an attempt to study how the energy absorbing device (TMD) can perform during earthquake.

2. BUILDING DESCRIPTION

Symmetrical plan building (G+4) has been considered for the present study, Design has been carried for the vertical load combination, which is having 2 bay along both X & Y axis along with 6m spacing in each grid, story height of 3m, grade of concrete=M25, poisons ratio as 0.15.

Numerical Formulation TMD:TMD can be modeled as spring mass system with three primary elements; a mass, a spring & a damper. Several authors has given the numerical stimulation technique in which Prof.Hartog's (1947) made remarkable mathematical formulation which is well known and it was adopted all over with certain limitations.

3. EARTHQUAKE TIME HISTORY

Earthquake time history file includes information such as origin time of earthquake which happened on that particular

station where we are recording responses along with longitude, latitude, magnitude, station height, depth, site classification based on shear wave velocity, maximum acceleration, duration of time, sampling rate. Usually acceleration data of Indian earthquake will be measured in terms of cm/sec². Some of the researchers are interested to converter this data in terms of acceleration due to gravity (g) i.e. 0.2g,0.3g,0.4g by amplifying recorded ground motion. Table 1 gives some insight knowledge into the ground motion data study.

Table-1: Time History Data File Information.

Latitude	27.6 N	28.6 N
Longitude	76.7 E	77.0 E
Depth (Km)	13	20.3
Magnitude	3.9	4.3
Region	ALWAR-RAJASTHAN	DELHI-HARYANA-BORDER-REGION
Station Code	ALW	REW
Station Height(m)	289	246
Site Class	A	C
Record Time	29.11.2006 05:41:05.554	25.11.2007 23:12:26.524
Sampling Rate	200. Hz	200. Hz
Record Duration	31.130 Sec.	33.530 Sec.
Direction	Vertical	Vertical
Max. Acceleration(cm/sec²)	51.705	-12.246

4. TIME HISTORY ANALYSIS

Time history analysis belongs to dynamic analysis. Time history analysis is performed by privies model analysis result by model superposition or by direct integration techniques with each increment in time step .Model superposition type can used to perform FNA in computer simulated model which requires very less disk space whereas Direct integration method is little cumbersome in case of time and requires very large disk space and time. So time history analysis kept away for usual daily design practice and restricted to research works.

5. RESULTS

Strictly speaking with sound knowledge of experiment data of devices, earthquake engineer should demonstrate detailed analysis and design with acceptable performance values of device. Then only a tuned mass damper or energy observing device can be used in earthquake resistant design.

TMD shows remarkable impact of use of passive-mechanical damping methods

5.1 Lateral Displacement

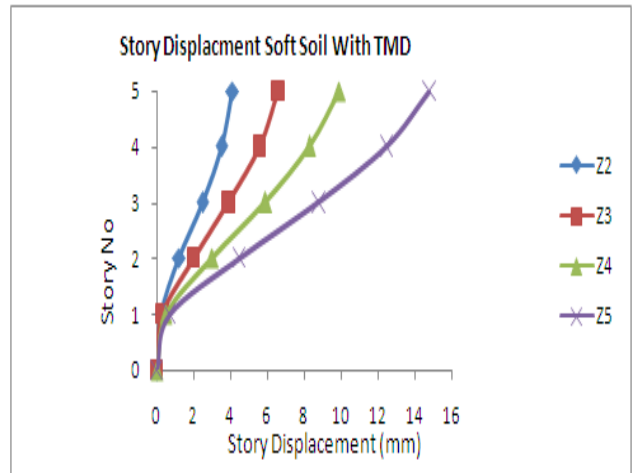


Chart -3: Zonal compression of lateral displacement for with TMD in soft soil

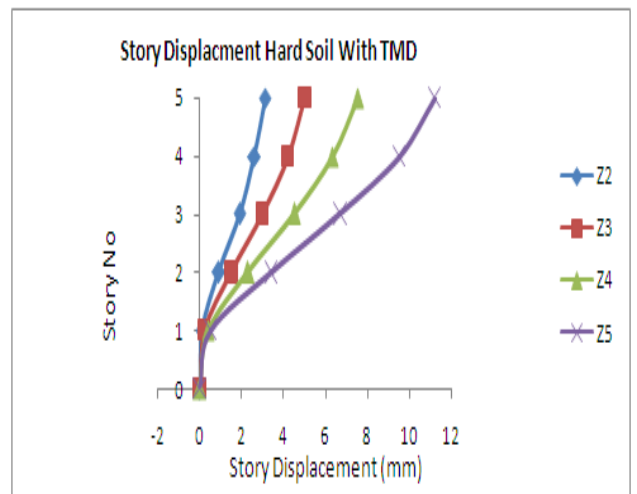


Chart -4: Zonal compression of lateral displacement for with TMD in hard soil

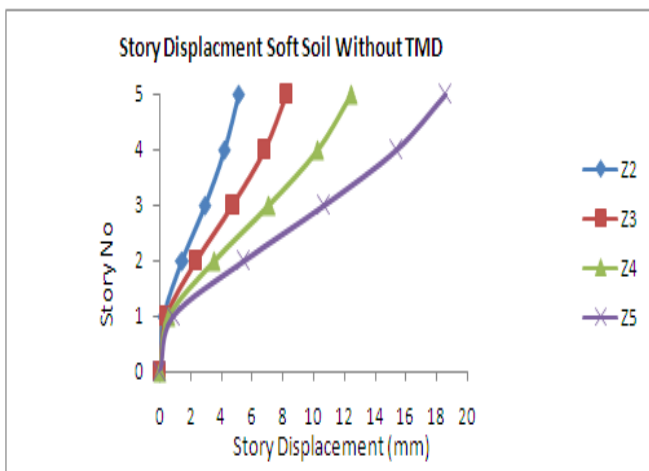


Chart -1: Zonal compression of lateral displacement for without TMD in soft soil

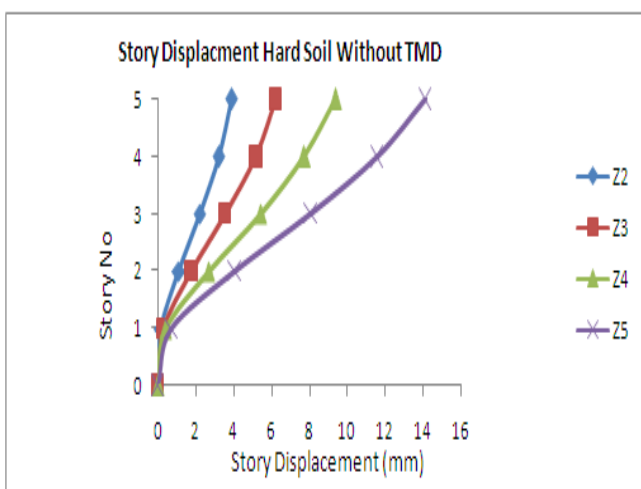


Chart -2: Zonal compression of lateral displacement for without TMD in hard soil

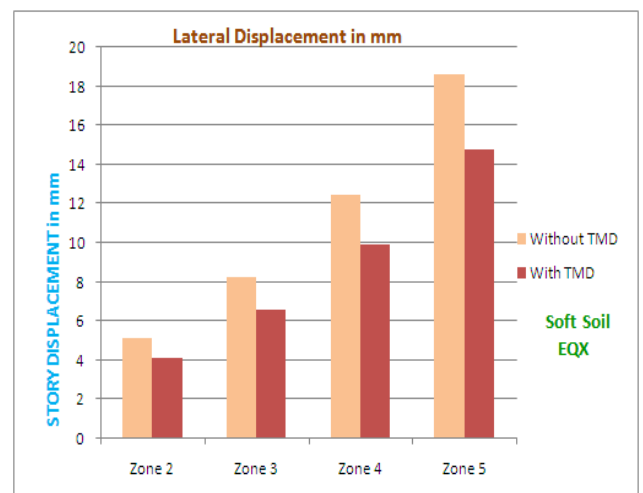


Chart -5: Compression of lateral displacement for with & without TMD in soft soil

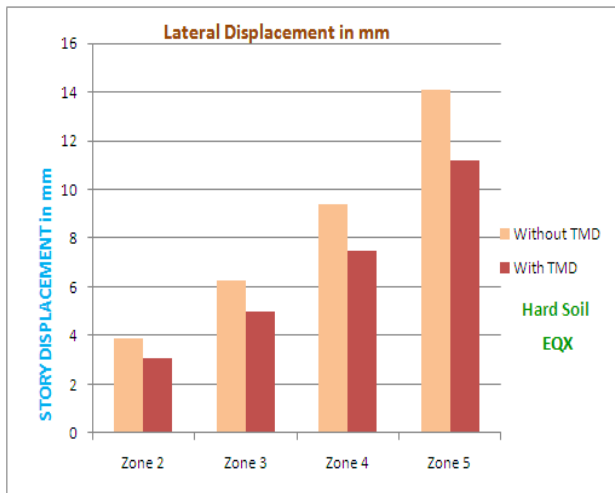


Chart -6: Compression of lateral displacement for with & without TMD in hard soil

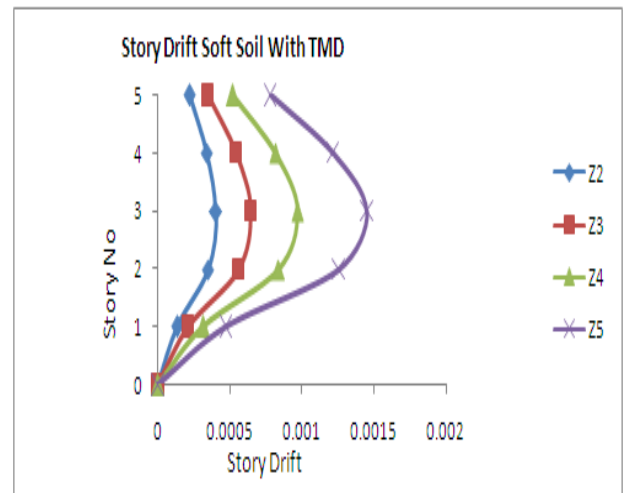


Chart -9: Zonal compression of Story Drift for with TMD in soft soil

5.2 Story Drift

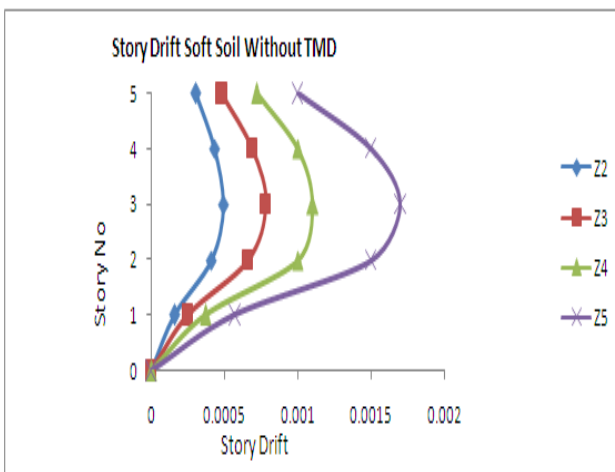


Chart -7: Zonal compression of Story Drift for without TMD in soft soil

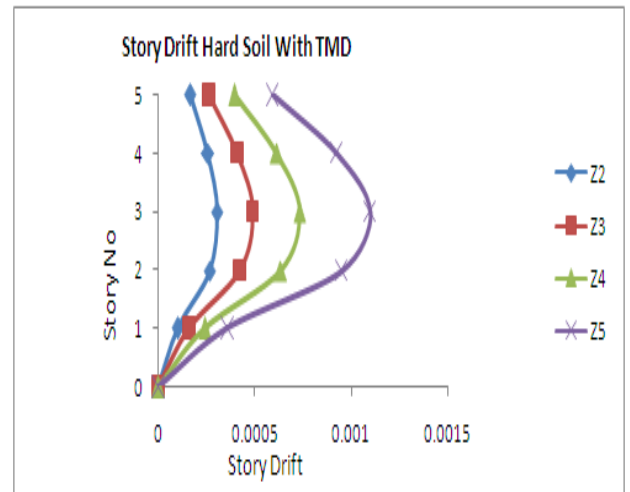


Chart -10: Zonal compression of Story Drift for with TMD in hard soil

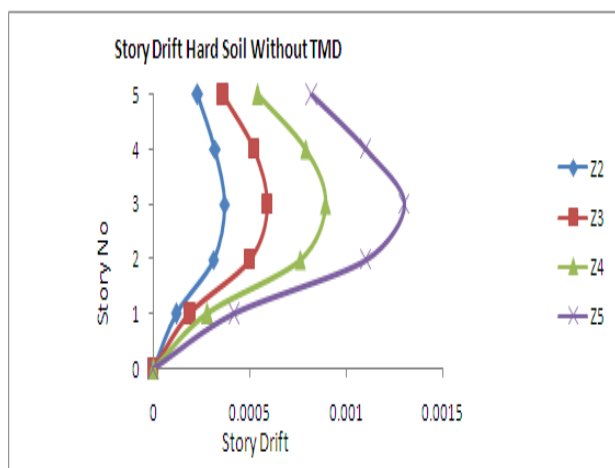


Chart -8: Zonal compression of Story Drift for without TMD in hard soil

5.3 Story Shear

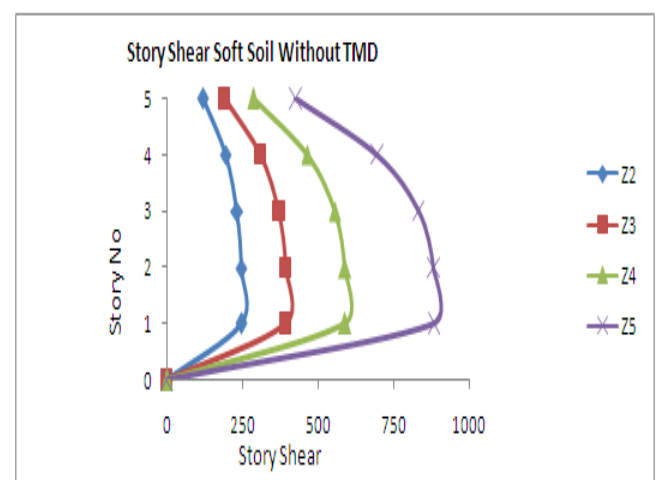


Chart -11: Zonal compression of Story Shear for without TMD in soft soil

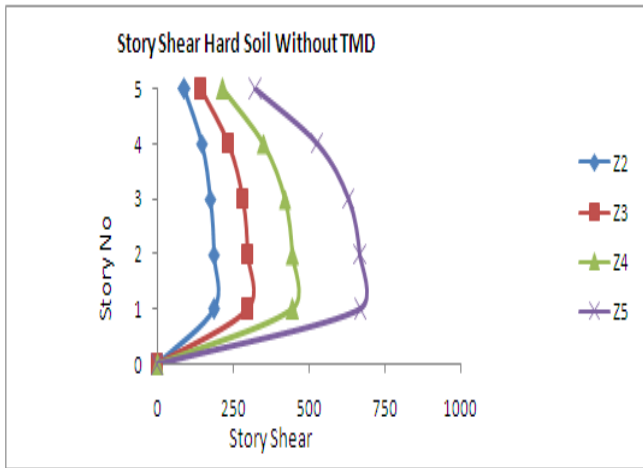


Chart -12: Zonal compression of Story Shear for without TMD in hard soil

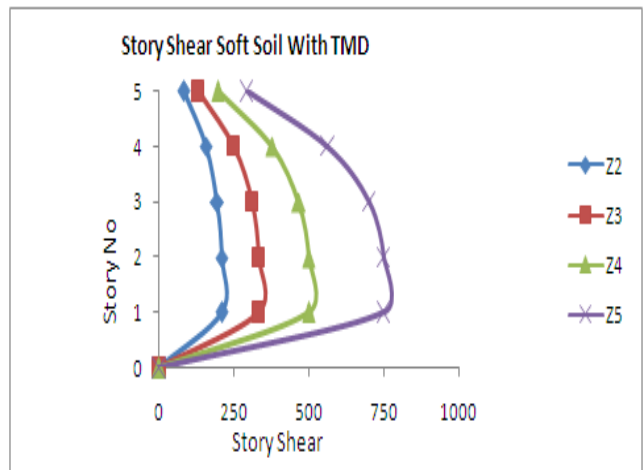


Chart -13: Zonal compression of Story Shear for with TMD in soft soil

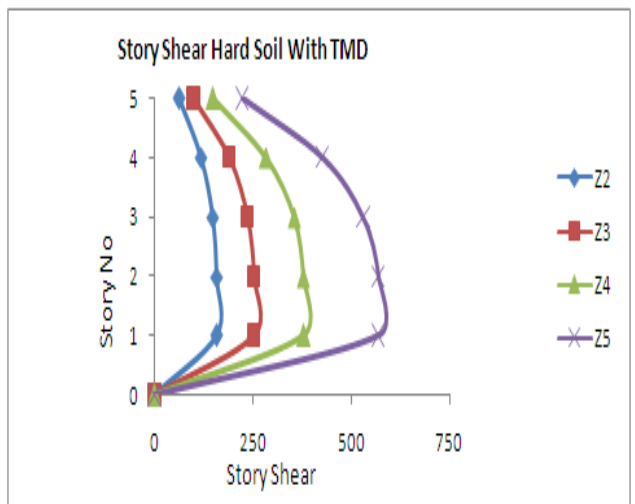


Chart -14: Zonal compression of Story Shear for with TMD in hard soil

5.4 Overturning Moment

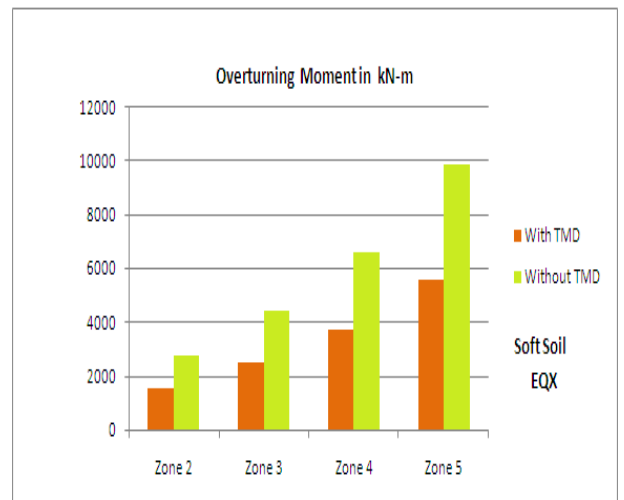


Chart -15: Compression of overturning moment for with & without TMD in soft soil

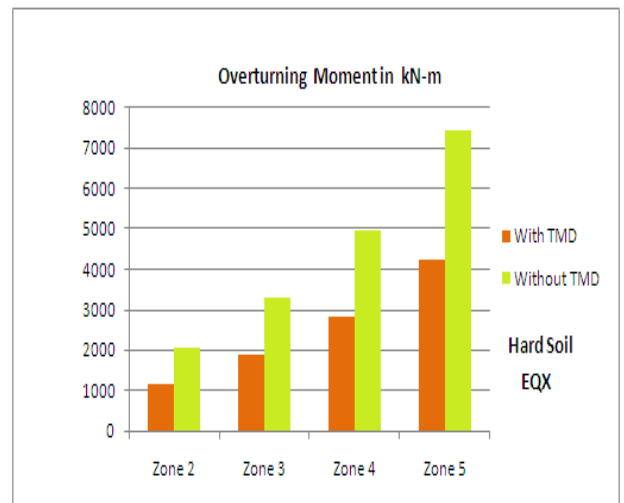


Chart -16: Compression of overturning moment for with & without TMD in hard soil

5.5 Time History Analysis

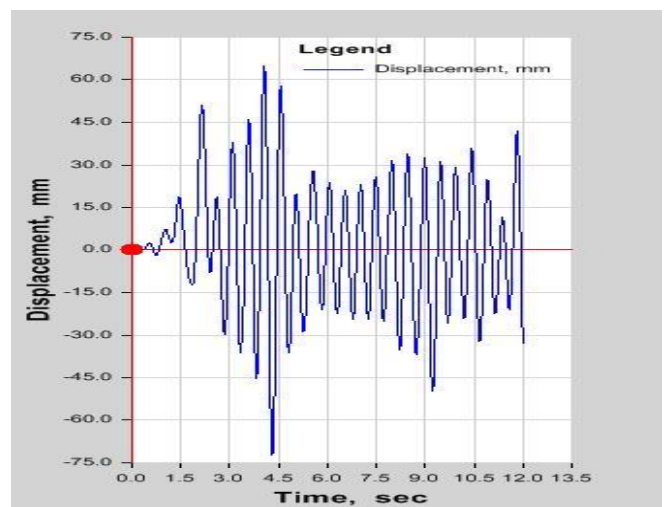


Chart -17: Roof displacement @ story 4 for with TMD in El-Centro Ground motion

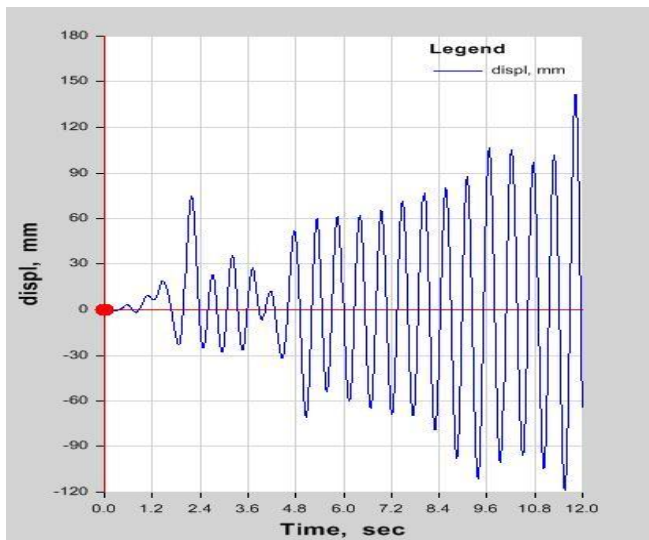


Chart -18: Roof displacement @ story 4 for without TMD in El-Centro Ground motion

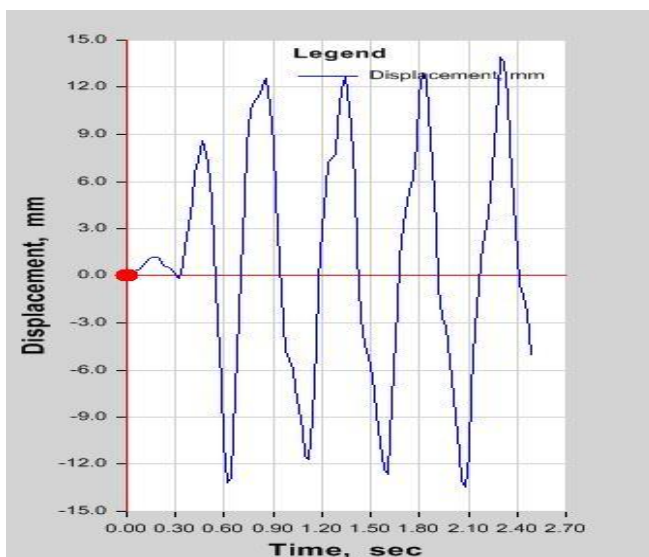


Chart -19: Roof displacement @ story 4 for with TMD in Yermo Ground motion

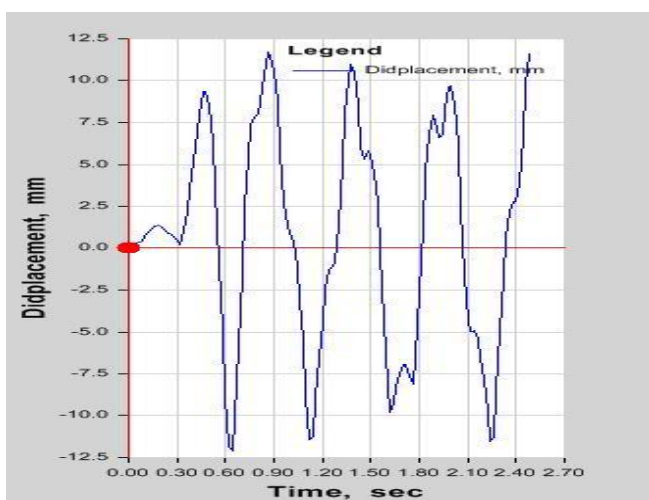


Chart -20: Roof displacement @ story 4 for without TMD in Yermo Ground motion

6. CONCLUSION

As in the study we considered lateral resisting system which are most essential in case of lifeline structure or major structures such as hospital and nuclear power station, which illustrates that more the number in combination of lateral resisting systems, the less in global collapse mechanism.

- 1) It's clear from above chart's that there is a cumulative increase in the values of story drift, Lateral displacement, Story shear & overturning moment in the order of zone 2 to zone 5 with hard soil.
- 2) Implementation of TMD system results into reduction in story drift, story shear, lateral displacement, overturning moment. This reduction enables the structure to behave as almost stiff. In this way we can reduce localized damage of non-structural & structural elements.

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