STUDY OF STRENGTH PROPERTIES OF CONCRETE BY USING MICRO SILICA AND NANO SILICA

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Abstract

Concrete is the most versatile material due to the persistent and continuous demands made on concrete, Engineers are continually pushing the limits to improve its performance with the help of innovative chemical admixtures and supplementary cementitious materials like fly ash, silica fume, granulated blast furnace slag and steel slag etc... The use of large quantity of cement produces increasing co_2 emissions and consequence the green house effect. A method to reduce the cement content in concrete mixes is the use of silica fume which is an amorphous (non-crystalline) polymorph of silicon dioxide, silica. It is an ultra fine powder collected as a byproduct of the silicon and ferrosilicon alloy production with an average particle diameter of 0.1 to 0.5 μ . The past investigations reveled that silica fume was an excellent pozzolanic material in producing High performance concrete (HPC).

Nano technology is one of the most promising areas of science. The use of nano materials in concrete is new revolution. Nano materials like nano silica, nano titanium oxide, carbon nano tubes, nano alumina etc... which are presently used in concrete to modify its strength properties. In the present study strength properties such as Compressive strength, split tensile strength and flexural strength of M_{40} and M_{50} grades of concrete with the use of micro silica (5%, 7.5%, 10%, 15%) and nano silica (1%, 1.5%, 2%, 2.5%) as partial replacement of cement were studied. It was found from the experimental study that concrete composites with superior properties can be produced using micro silica, nano silica and combination of micro silica and nano silica.

Keywords: Colloidal Nano Silica (NS), Silica Fume (SF), Compressive strength (CS), Split tensile strength (ST) and

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Flexural strength (FS).

1. INTRODUCTION

Concrete is the most widely used construction material in the world. In recent years, researchers have focused on the improvement of concrete quality regarding its mechanical and durability properties. These can be achieved by the application of the supplementary cementitious materials.

Out of these supplementary cementitious materials, silica fume is the one of the waste materials that is being produced in tones of industrial waste per year in our country. The first testing of silica fume in Portland-cement-based concretes was carried out in 1952. The biggest drawback to exploring the properties of silica fume was a lack of material to experiment with. Early research used an expensive additive called fumed silica, an amorphous form of silica made by combustion of silicon tetrachloride in a hydrogen-oxygen flame. Silica fume on the other hand, is a very fine pozzolanic material. It is a byproduct of producing silicon metal or ferrosilicon alloys. One of the most beneficial uses of silica fume is in concrete. Because of its chemical and physical properties; it is a very reactive pozzolanic material. Concrete containing silica fume has very high strength and is very durable.

Recently Nano Technology has been introduced in Civil Engineering applications. One of the most used nano material is Nano Silica (NS). This is the first Nano product that has replaced the micro silica. The advancement made by the study of concrete at nano scale has proved that nano silica is much better than silica fume used in conventional concrete. Nano silica possess more pozzolanic nature, it has the capability to react with the free lime during the cement hydration and forms additional C-S-H gel which gives strength, impermeability and durability to concrete.

Verma ajay et al, (2012)^[1] have studied the effect of micro silica and the strength of concrete with ordinary Portland cement. They observed that silica fume increases the strength of concrete and reduces capillary pores. **Dilip Kumar Singha Roy (2012)**^[2] has investigated on the strength parameters of concrete made with partial replacement of cement by SF. **T.Shanmugapriya (2013)**^[3] studied the influence of silica fume on M_{60} concrete and found that 7.5% of silica fume replacement increases the maximum compressive strength, split tensile strength and flexural strength.

Ji (2005)^[4] studied the water permeability resistant behavior and micro structure of concrete with NS and observed that NS concrete has a better water resistant permeability than ordinary concrete. **Ye Qing et al**, (2007)^[5] studied the influence of silica fume and nano silica individually on fresh concrete and hardened concrete and found that consistency and setting times were different for NS and SF. NS makes cement paste thicker and accelerated the hydration process which improves the bond strength and compressive strength when compared with that of SF in concrete. **Byung-Wan** (2007)^[6] studied the properties of cement mortar with NS particles and reported the importance of addition of NS on mechanical properties. Lin et al. (2008) ^[7] have observed the effect of NS addition on permeability and compressive strength of fly ash cement mortar. From the pore analysis study, it was reported that the relative permeability and pore sizes of concrete were decreased, whereas the compressive strength increased by adding more NS. Yazdi et al (2009)^[8] investigated the effect of NS on high volume fly ash concrete [HFC], and found that due to the low pozzolanic reactions of fly ash, early strength of HFC reduced considerably, but with the addition of NS promoted the pozzolanic activity reaction which enabled the enhancement of strength of HFC, especially in the early ages. Sadrmontazi Barzegar (2010)^[9] concluded that the properties of self compacting concrete with and without Rice Husk Ash [RHA], an agro-industry waste, and exhibited improvement in the physical and mechanical properties of concrete with addition of NS. A. Siva Sai, B.L.P. Swami, B.SaiKiran (2013) ^[10] have observed the mechanical properties of M₆₀ and M₇₀ grade concrete with micro silica and in combination with colloidal nano-silica. They found that concrete composites with superior properties can be produced with the combination of microsilica and nano-silica. Mohammad Reza Zamani Abyaneh, et al (2013)^[11] have found that the concrete produced with Micro-SiO₂ and Nano-SiO₂ show higher degrees of quality in their compressive strength than the concrete which only have Micro-SiO₂ in their mixtures. Specimens with 2% Nano-SiO₂ and 10% Micro-SiO₂ had less water absorption and more electrical resistance.

2. MATERIALS USED AND THEIR

PROPERTIES

In this present investigation materials used are Cement, Fine aggregate, Coarse aggregate, Silica Fume, Nano silica, Super plasticizer.

2.1 Cement:

Ultra tech cement of ordinary Portland cement (OPC) of 53 Grade was used which satisfies the requirements of IS: 12269-1987. The properties of cement are shown in Table 1.

Sno	Property of cement	Results	
1	Normal Consistency	31%	
2	Initial setting time	120 min	
3	Final setting time	250 min	
4	Specific gravity	3.11	
5	Compressive strength of	55.6 mpa	
	cement at 28 days		

 Table 1 Properties of cement

2.2 Aggregate:

Fine Aggregate: locally available sand collected from Tungabhadra River bed was used. The sand was conforming to zone II as per IS: 383-1987. The properties of fine aggregate are shown in Table 2.

 Table 2 Properties of Fine aggregate

Sno	Properties	Results
1	Bulk density, kg/m ³	1650
2	Specific gravity	2.68
3	Fineness modulus	2.81
4	Free surface moisture (%)	2.0

Coarse Aggregate: The crushed aggregate was used from the local quarry. In this experiment the aggregate was used of 20mm down and tested as per IS: 2386-1963(I, II, III) specification. The properties of coarse aggregate are shown in Table 3.

Table 3 Propertie	s of Coarse aggregate
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Sno	Property	Results
1	Maximum nominal size	20mm
2	Bulk density (kg/m ³)	1800
3	Specific gravity	2.75
4	Fineness modulus	4.6

2.3 Silica Fume:

The silica fume was used in these experiments conforms to ASTM C 1240 and IS 15388:2003. The silica fume is extremely fine particle, which exists in white color powder form. Silica fume has been procured from Astrra chemicals Ltd-Chennai. The properties of silica fume is shown in Table 4

Sl. No.	Properties	Results
1	Form	Ultra fine amorphous
1	1.0111	powder
2	Colour	White
3	Specific gravity	2.63
4	Pack Density	0.76 gm/cc
5	Specific surface	$20 \text{ m}^2/\text{g}$
6	Particle size	15µm
7	Sio ₂	99.89%

 Table 4 Properties of silica fume.

2.4 Nano Silica:

CemSyn®-XFX is a series of silica based binders /fillers obtained from Bee-chem: Chemicals Ltd., Kanpur. The properties of nano silica are shown in table-5

 Table 5 Properties of nano silica

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State	Dispersed in water	
Active nano Content	40.00-41.50	
(%w/w)		
Ph (20° C)	9.0-10.0	
Specific gravity	1.30-1.32	
Particle size	5-40 nm	

2.5 Super Plasticizers:

Fosroc Aura mix 400 was used for M_{40} and M_{50} Grade of concrete. The properties of super plasticizer is shown in table6

properties			
Appearance	Light yellow coloured liquid		
Ph	Minimum 6.0		
Volumetric mass	1.09 kg/ liter		
@ 20°C			
Chloride content	Nil		
Alkali content	Typically less than 1.5 g Na ₂ o		
	equivalent liter of admixture		

3. EXPERIMENTAL PROGRAMME

The experimental program was designed to compare the mechanical properties i.e. compressive strength, split tensile strength, and flexural strength of high strength concrete with M_{40} and M_{50} grade of concrete and with different replacement levels of ordinary Portland cement (ultra tech cement 53 grade) with silica fume (5%, 7.5%, 10% and 15%) and nano silica (1%, 1.5%, 2% and 2.5%). Strength properties of M_{40} and M_{50} grade concretes were also studied for combination of optimum replacement levels of SF (7.5%) and NS (2%).

3.1 Mix Proportions:

Two concrete mixes were designed to a compressive strength of M_{40} and M_{50} grades with water cement ratio of 0.36 and 0.33 respectively as per IS code 10262-2009. In the both cases, the cement was replaced with SF by (5%, 7.5%, 10% and 15%) and NS by (1%, 1.5%, 2% and 2.5%) the proportions of constituent materials for two Mixes are presented in table 7

SL.	Materials	Quantities in Kg/m ³	
No		M ₄₀ Grade	M ₅₀ Grade
1	Cement	403	439.64
2	Water	145.08	145.08
3	Fine aggregate	682.670	659.969
4	Coarse aggregate	1252.046	1242.802
5	Water cement ratio	0.36	0.33

 Table 7 Mix proportion of concrete

The specimens of standard cubes (150mmx150mmx150mm), standard cylinders of (150mm Dia x300mm height) and standard beams of (100mmx100mmx500mm) were cast with various percentage replacements of SF and NS. Compression testing machine (CTM) was used to test 28 days compressive strength and split tensile strength of specimens. Universal Testing Machine (UTM) was used to test 28 days flexural strength of specimens

4. RESULTS AND DISCUSSIONS

4.1 Mechanical Properties:

4.1.1 Compressive Strength:

The compressive strength M_{40} and M_{50} grade concrete, SF concrete and NS concrete at the age of 28 days is presented in table 8.

There is a significance improvement in the strength of concrete because of high pozzolanic nature of silica fume and nano silica and their filling ability. Compressive strength of two mixes M_{40} and M_{50} at 28 days age, with replacement of SF was increased gradually up to an optimum replacement level of 7.5% and then decreased. The maximum 28 days cube strength of M_{40} grade with 7.5% of silica fume was 61.24N/mm² and of M_{50} grade with 7.5% SF was 69.09 N/mm².

Compressive strength of M_{40} and M_{50} at 28 days age with replacement of NS was increased gradually up to an optimum replacement level of 2% and then decreased. The maximum 28 days cube compressive strength of M_{40} grade with 2% NS was 59.61 N/mm² and of M_{50} grade with 2% NS was 69.72 N/mm².

The compressive strength of M_{40} grade concrete with partial replacement of cement by 7.5% SF shows 23.569% and of M_{50} grade with 7.5% replacement shows 22.534%. The compressive strength of M_{40} grade concrete with partial replacement of cement by 2% NS shows 20.278% and of M_{50} grade with 2% replacement shows 22.236%.

Compressive strength of M_{40} & M_{50} grades were also studied with the combination of SF at 7.5% and NS at 2% which results in a marginal improvement in strengths over respective optimal replacement levels of SF (7.5%) and NS (2%).Figure 1 and Figure 2 shows the variation of compressive strength of M_{40} & M_{50} grade with SF and NS replacements.

4.1.2 Flexural Strength:

The flexural strength of M_{40} and M_{50} grade concrete, SF concrete and NS concrete at the age of 28 days is presented in table 9.

Flexural strength of two mixes M_{40} and M_{50} at 28 days age, with replacement of SF was increased gradually up to an optimum replacement level of 7.5% and then decreased. The maximum 28 days beam strength of M_{40} grade with 7.5% of silica fume was 4.160 N/mm² and of M_{50} grade with 7.5% SF was 4.560 N/mm²

Flexural strength of M_{40} and M_{50} at 28 days age with replacement of NS was increased gradually up to an optimum replacement level of 2% and then decreased. The maximum 28 days beam flexural strength of M_{40} grade with 2% NS was 4.45 N/mm² and of M_{50} grade with 2% NS was 4.71 N/mm²

The Flexural strength of M_{40} grade concrete with partial replacement of cement by 7.5% SF shows 9.186% and of M_{50} grade with 7.5% replacement shows 9.352%. The Flexural strength of M_{40} grade concrete with partial replacement of cement by 2% NS shows 16.797% and of M_{50} grade with 2% replacement shows 13%.

Flexural strength of M_{40} & M_{50} grades were also studied with the combination of SF at 7.5% and NS at 2% which results in a marginal improvement in strengths over respective optimal replacement levels of SF (7.5%) and NS (2%).Figure 3 and Figure 4 shows the variation of Flexural strength of M_{40} & M_{50} grade with SF and NS replacement results.

4.1.3 Split Tensile Strength:

The Split tensile strength M_{40} and M_{50} grade concrete, SF concrete and NS concrete at the age of 28 days is presented in table 10.

Split tensile strength of two mixes M_{40} and M_{50} at 28 days age, with replacement of SF was increased gradually up to an optimum replacement level of 7.5% and then decreased. The maximum 28 days cylinder strength of M_{40} grade with 7.5% of silica fume was 3.960 N/mm² and of M_{50} grade with 7.5% SF was 4.12 N/mm²

Split tensile strength of M_{40} and M_{50} at 28 days age with replacement of NS was increased gradually up to an optimum replacement level of 2% and then decreased. The maximum 28 days cylinder Split tensile strength of M_{40} grade with 2% NS was 4 N/mm² and of M_{50} grade with 2% NS was 4.320 N/mm²

The Split tensile strength of M_{40} grade concrete with partial replacement of cement by 7.5% SF shows 21.472% and of M_{50} grade with 7.5% replacement shows 17.613%. The Split tensile strength of M_{40} grade concrete with partial replacement of cement by 2% NS shows 22.7% and of M_{50} grade with 2% replacement shows 23.322%.

Split tensile strength of M_{40} & M_{50} grades were also studied with the combination of SF at 7.5% and NS at 2% which results in a marginal improvement in strengths over respective optimal replacement levels of SF (7.5%) and NS (2%).Figure 5 and Figure 6 shows the variation of split tensile strength of M_{40} & M_{50} grade with SF and NS replacement results.

 Table: 8 shows the compressive strength of concrete at 28 days

SNO	% Silica	% Nano	Compressiv in N/mm ²	e Strength
5110	Fume	Silica	M ₄₀ Grade	M ₅₀ Grade
1	0	0	49.56	57.037
2	5	0	57.185	61.000
3	7.5	0	61.241	69.890
4	10	0	48.741	44.580
5	15	0	46.222	42.074

6	0	1	54.111	62.267
7	0	1.5	55.250	65.790
8	0	2	59.610	69.720
9	0	2.5	47.000	51.415
10	7.5	2	62.350	71.5

Table: 9 shows the Flexura	l strength of concrete at 28 days
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SNO	% Silica	% Nano Silica	Flexural Strength in N/mm ²	
	Fume		M ₄₀ Grade	M ₅₀ Grade
1	0	0	3.81	4.17
2	5	0	4	4.280
3	7.5	0	4.160	4.560
4	10	0	3.760	3.985
5	15	0	3.960	4.053
6	0	1	4	4.250
7	0	1.5	4.2	4.590
8	0	2	4.450	4.710
9	0	2.5	3.80	4
10	7.5	2	4.53	4.840

 Table: 10 shows the split tensile strength of concrete at 28 days

SNO	% Silica	% Nano Silica	Split Tensile Strength in N/mm ²	
	Fume		M ₄₀ Grade	M ₅₀ Grade
1	0	0	3.26	3.503
2	5	0	3.840	4
3	7.5	0	3.960	4.120
4	10	0	3.010	3.210
5	15	0	3.270	3.380
6	0	1	3.748	3.981
7	0	1.5	3.819	4
8	0	2	4.	4.320
9	0	2.5	3.253	3.680
10	7.5	2	4.1	4.380

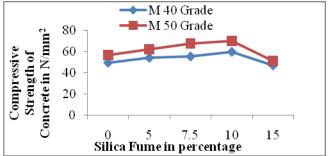


Fig: 1 Variation of compressive strength with silica fume

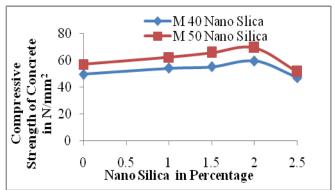


Fig: 2 Variation of compressive strength of concrete with nano silica

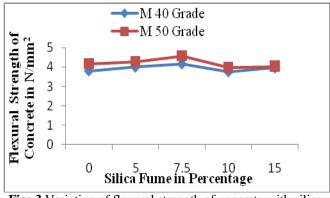


Fig: 3 Variation of flexural strength of concrete with silica fume

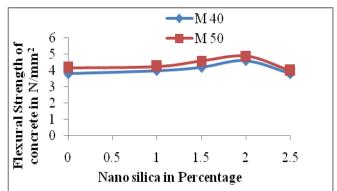
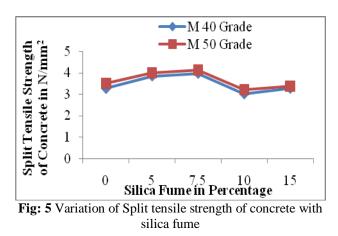


Fig: 4 Variation of Flexural strength of concrete with nano silica



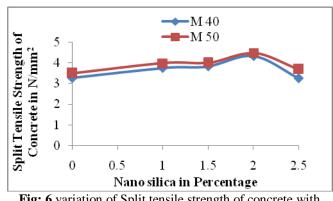


Fig: 6 variation of Split tensile strength of concrete with nano silica

5. CONCLUSIONS

Based on experimental results the following conlusions are drawn

- 1. Cement replacement up to 7.5% with SF and up to 2% with NS, leads to increasing compressive strength, split tensile strength and flexural strength for both M_{40} and M_{50} grade. Beyond 7.5% of SF and 2% of NS there is decreasing in compressive strength, split tensile strength and flexural strength for both M_{40} and M_{50} mixes.
- 2. The maximum replacement level of silica fume is 7.5% and nano silica is 2% for both M_{40} and M_{50} grade concrete.
- 3. The percentage increase in compressive strength, split tensile strength and flexural strength of concrete with combination of SF at 7.5% and NS at 2% is (25.807%, 25.766% and 18.9%).for M_{40} grade and (25.357%, 25.035% and 16.067%) for M_{50} grade concrete which is More when compared to normal concrete of M_{40} and M_{50} grades.

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