

# AN AXISYMMETRIC BENDING AND SHEAR STRESS ANALYSIS OF OF FUNCTIONALLY GRADED CIRCULAR PLATE BASED ON UNCONSTRAINED THIRD ORDER SHEAR DEFORMATION THEORY VIA DIFFERENTIAL QUADRATURE METHOD

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## Abstract

*In this study, based on the unconstrained third order shear deformation theory (UTSDT), numerical analysis of an axisymmetric bending and stresses of circular plate are investigated. The material properties are considered to graded through the thickness of the verticlecoordinate, and follow a simple power of volume fraction of the constituents.governing equations are derived and DQM is used as an efficient numerical method for solving the differential equations.Two types of boundary conditions under the influence of the bending and body force are studied. The validation of the results is done by a comparison with another study ,which available in the literature and found good agreement between two studies.*

**Index Terms:***bending, shearstress, circularplate, UTSDT, GDQM.*

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## 1. INTRODUCTION

Thick and thin Circular disk in structured components plays a major role in engineering applications related to this area is the static analysis the types of plates which are notably crucial in their design ranging from automotive railway brake systems to disks which constitute vital components particularly in turbo machines. Functionally, graded materials (FGMs) were first introduced in 1989 [1] whereby a number of researchers, because interested to study them .

In the past decade , many of the studies which carried out on the FGMs disks concentrated on the conventional plate and the first order shear deformation theories . The conventional plate theory (CPT) furnishes accurate and reliable analysis for this plate . As the disk thickness increases CPT over predicts stresses response, because the transverse shear deformation and rotary inertia effects are neglected .So there a number of shear deformation theories used to analyze moderately thick plate , first order theory and third order theory were developed to incorporate the shear deformation effects , in the first order shear deformation theory (FSDT), the constant shear stress condition through thicknesses violates the statically condition of zero shear stress at the free surface . So its need for shear correction factor to modify the shear forces .The third order shear deformation theory (TSDT )predicts parabolic variation of shear stress through the thickness. Although the use of higher order plate theory leads to more an accurate prediction of the global response quantities such as shear forces , deflections strain and stresses , it requires much

computation effort . Furthermore the use of the (TSDT) by Reddy is constrained , because it considers the shear stress vanishes on the top and bottom surfaces of the plate , but this limitation is solved by the unconstrained third order shear deformation theory (TSDT) by Leuny [2].

In past decades several studies published on the static analysis of FGMs circular disks Reddy et al. [3] Study relates to axisymmetric bending of functionally graded circular and annular plates whereby the first order shear deformation plate theory was used. Ma and Wang [4] analyzed further by discussing the relationship between axisymmetric bending and buckling solutions of FGM circular plates. Third-order plate theory and classical plate theory were demonstrated and discussed in detail in their study. In addition, asymmetric flexural vibration and an additional stability analysis of FGM circular plates was included in thermal environment by using finite element techniques presented by Prakash and Ganapathi [5]. Also, Three-dimensional free vibration of functionally graded annular plates whereby boundary conditions were different using a Chebyshev-Ritz method was also studied by Dong [6]. Malekzadeh et al. [7] Also showed how in thermal environment in-plane free vibration analysis of FGM thin-to-moderately thick deep circular arches. Third-order shear deformation theory was used by Saidi et al. [8] To analyze axisymmetric bending and buckling of thick functionally graded circular plates. Subsequently, fourth-order shear deformation theory was researched by Sahraee and saidi [9] to study axisymmetric bending of thick functionally graded

circular plates. Besides this, Sepahi et al. [10] Analyzed the effects of big deflection of thermo-mechanical loaded annular FGM plates on nonlinear elastic foundation using the differential quadrature method. Geometrically nonlinear post-buckling of an imperfect circular FGM plate was studied by Li et al. [11] who found both mechanical load and transverse non-uniform temperature rise. A study conducted by Malekzadeh et al. [12] resulted in three-dimensional free vibration of thick functionally graded annular plate in thermal environment by differential quadrature technique. An investigation of nonlinear analysis of functionally graded circular plates was conducted by Nosier and Fallah [13] It was related to asymmetric transverse loading, according to the first-order shear deformation plate theory based on von Karman non-linearity. In addition, Sburlati and Bardella [14] studied three-dimensional elasticity solution of functionally graded thick circular plates. Correspondingly, Golmakani and Kadhodayan [15] studied axisymmetric nonlinear bending analysis of annular functionally graded plate. The study used third-order shear deformation theory. A precise closed form answer for free vibration of circular and annular moderately thick functionally graded plates of first-order shear deformation theory was studied by Hosseini-Hashemi et al. [16] Nie and Zhong's study [17] was on frequency analysis of multi-directional functionally graded annular plates using state space differential quadrature method. It was based on the three-dimensional theory of elasticity. It must be noted that direct displacement method was conducted by Yan et al. [18] with the aim to represent the axisymmetric bending of FG circular plates under transverse loads that were arbitrary. Another theory using Mindlin's plate theory about free vibration was investigated by Ebrahimi et al [19]. This study was concerned about moderately thick shear deformable annular functionally graded by Pilate. The effects of coupling between in-plane and out-of-plane vibrating modes of smart functionally graded circular/annular plates was examined by Hashemi et al. [20]. Nonlinear bending and post-buckling of a functionally graded circular plate was examined by Ma and Wang [21] whereby the conditions were mechanical and thermal loading. Bayat et al [22] used first-order shear deformation theory to study the thermal elastic response of rotating disk with small and large deflections, and presented the results with analytical solutions.

Viola et al. [23] Used a 2D unconstrained third order shear deformation theory (UTSDT) in static analysis of moderately thick functionally graded cylindrical shells subjected to mechanical loadings. Also Viola et al. [24] Employed (UTSDT) for analyzing the dynamic behaviour of completely doubly-curved laminated shells and panels .

It is clear from the above literature most of the studies which carried out on the static analysis of the circular disks based on the first order shear deformation theory and few of them based on the third order deformation theory which is solved analytically for limited boundary conditions. On the other

hand the numerical technique presenting static analysis for the circular disks based on high unconstrained third order shear deformation theory is quite poor .As well as , the use of the shear function model which is used by [24] can be applied to the displacement field of the circular disk .

In the present study, unconstrained third order shear deformation theory is used for axisymmetric static analysis of functionally graded clamped and rotating circular plate .The circular plate is subjected to two types of loading , bending and body force . The mechanical properties are assumed to be graded in the thickness direction according to a simple power law distribution in terms of the volume fraction of the constituent . By the principle of minimum total energy, the governing equations of equilibrium are obtained according to the unconstrained third order shear deformation theory . By employing the differential quadrature method as a simple but accurate and fast convergent method to discretize the equilibrium equations and to implement the boundary conditions . The effects of body force parameters ,the material constant, and the geometric parameters of circular plate on the stresses and deflection response are studied in detail.

## 2. PROBLEM STATEMENT

Consider a FG circular disk with thickness(  $h$  ) and radius (  $a$  ) , axisymmetric with respect to the  $z$ -axis as shown in fig.1 and it is subjected to uniform transverse pressure in case of clamped condition, while in case of roller support condition, it is subjected to both uniform pressure and body force

The mid-plane of the plate refers to the cylindrical coordinate system  $(r, \theta, z)$  in the radial ,circumferential and axial directions respectively.

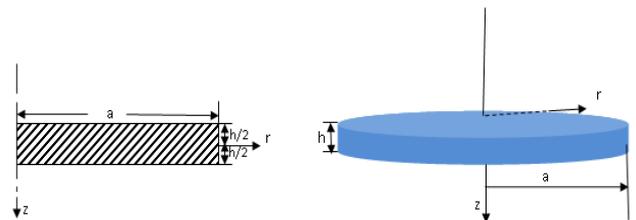


Figure 1: FG circular disk with thickness (  $h$  ) and radius (  $a$  )

### 2. 1- Mechanical Properties Of FG Circular Plate Types

Typically FGMs are made of a mixture of the two constituents . In this research it is assumed that the FGMs are made of a mixture of ceramic and metal constituents.the material properties of the FG plate vary continuously and smoothly in thickness direction  $z$  and are functions of volume fraction of constituent materials

$$P(z) = (P_m - P_c)V_m + P_c \quad (1)$$

Where:

$P(z)$ : material property at location  $z$  through thickness.  $m$  and  $c$  denotes the metallic and ceramic constituents respectively.

$V_m$ : volume fraction of metal

$$V_m = \left( \frac{h-2z}{2h} \right)^p \quad (2)$$

Where:

$z$ : thickness coordinates ( $-h/2 \leq z \leq h/2$ )  
 $p$ : material constant.

As the material constant is equal to zero ( $p=0$ ), or equal to infinity ( $p=\infty$ ), the homogeneous isotropic material is obtained as a special case of functionally graded material. In fact, from equation (2) it is possible to obtain:

$$p = 0 \rightarrow V_m = 1, V_c = 0 \rightarrow p(z) = p_m$$

$$p = \infty \rightarrow V_m = 0, V_c = 1 \rightarrow p(z) = p_c$$

Fig.2 shows that material profile through the FG plate for various values of  $p$ .

According to relation (1), the elastic modulus  $E$  and density  $\rho$  to be varied according to the above equation. And Poisson's ratio  $\nu$  is assumed to be constant.

$$E(z) = (E_m - E_c)V_m + E_c \quad (3)$$

$$\rho(z) = (\rho_m - \rho_c)V_m + \rho_c \quad (4)$$

### 3. GOVERNING EQUATIONS

Based on the unconstrained third-order shear deformation theory (UTSDT), displacement field in the cylindrical coordinate system can be written as:

$$U(r, z) = u(r) + f(z)\phi_1(r) + g(z)\phi_2(r) \quad (5)$$

$$w(r, z) = w(r) \quad (6)$$

Where:  $u, w$  are the displacements of points on the middle plane ( $z=0$ ) in the radial and vertical direction respectively.

$\phi_1$ : small transverse normal rotation about the  $\theta$ -axes

$\phi_2$ : small transverse normal higher order rotation about  $\theta$ -axes

$f(z), g(z)$  are shear functions.

From the previous [23], the displacement field has been improved by taking into consideration shear functions along the thickness. Indeed, the model for the shear function in this study has taken from previous work [24].

Strain-displacement relations:

$$\epsilon_{rr} = \frac{du}{dr} + (z - \alpha z^3) \frac{d\phi_1}{dr} - \alpha z^3 \frac{d\phi_2}{dr} \quad (7a)$$

$$\epsilon_{\theta\theta} = \frac{1}{r}u + (z - \alpha z^3) \frac{1}{r}\phi_1 - \alpha z^3 \frac{1}{r}\phi_2 \quad (7b)$$

$$\gamma_{rz} = (1 - 3\alpha z^2)\phi_1 - 3\alpha z^2\phi_2 + \frac{dw}{dr} \quad (7c)$$

$$\gamma_{r\theta} = 0, \gamma_{\theta z} = 0, \epsilon_z = 0$$

Stress-strain relations

$$\begin{Bmatrix} \sigma_r \\ \sigma_\theta \\ \tau_{rz} \end{Bmatrix} = \begin{bmatrix} Q_{11} & Q_{12} & 0 \\ Q_{12} & Q_{22} & 0 \\ 0 & 0 & Q_{66} \end{bmatrix} \begin{Bmatrix} \epsilon_r \\ \epsilon_\theta \\ \gamma_{rz} \end{Bmatrix} \quad (8)$$

Where:

$$Q_{11} = Q_{22} = \frac{E(z)}{2(1+\nu^2)}, Q_{12} = \nu Q_{11}$$

$$Q_{66} = \frac{E(z)}{2(1+\nu)}$$

The total potential energy of circular plate:

$$\Pi = U + V_f \quad (9)$$

$$U = \int_{-h/2}^a \int_{-h/2}^a 2\pi [\sigma_r \epsilon_r + \sigma_\theta \epsilon_\theta + \tau_{rz} \gamma_{rz}] r dz dr \quad (10)$$

$$V_f = -2\pi \int_0^a (\rho_1 r^2 \omega^2 \delta u + \rho_2 r^2 \omega^2 \delta \phi_1 - \alpha \rho_3 r^2 \omega^2 \delta \phi_1 - \alpha \rho_3 r^2 \omega^2 \delta \phi_2 + r q \delta w) dr \quad (11)$$

Where :

$\Pi, U, V_f$  are the total potential energy , strain energy and the potential energy of the body force and pressure load respectively.

By the principle of minimum total energy  $\delta \Pi = 0$

$$\int_0^a \left\{ \begin{aligned} & \left( -\frac{d}{dr}(rN_r) + N_\theta - \rho_1 r^2 \omega^2 \right) \delta u + \\ & \left( -\frac{d}{dr}(rM_r) + \alpha \frac{d}{dr}(rp_r) + M_\theta - \alpha p_\theta + r\phi_r - \right. \\ & \left. 3\alpha r R_r + \right. \\ & \left. (\alpha \rho_3 - \rho_1) r^2 \omega^2 \right) \delta \phi_1 + \\ & \left( \alpha \frac{d}{dr}(rp_r) - \alpha p_\theta - 3\alpha r R_r + \alpha \rho_3 r^2 \omega^2 \right) \delta \phi_2 - \\ & \left( \frac{d}{dr}(r\phi_r) + r q_r \right) \delta w \end{aligned} \right\} dr = 0 \quad (12)$$

Where:

$N_r, N_\theta$  :Stress resultants

$M_r, M_\theta$  :Stress couples

$P_r, P_\theta$  :higher order stress couples

$\phi_r$  :transverse shear resultant

$R_r$  : higher order shear resultant

$\rho_1, \rho_2, \rho_3$  : constant proportional to the mean ,first and third moment of the density along the thickness.

$$(N_r, M_r, P_r) = \int_{-h/2}^{h/2} \sigma_r(1, z, z^3) dz \quad (16a)$$

$$(N_\theta, M_\theta, P_\theta) = \int_{-h/2}^{h/2} \sigma_\theta(1, z, z^3) dz \quad (16b)$$

$$(\phi_r, R_r) = \int_{-h/2}^{h/2} \tau_{rz}(1, z^2) dz \quad (16c)$$

$$(\rho_1, \rho_2, \rho_3) = \int_{-h/2}^{h/2} \rho(z)(1, z, z^3) dz \quad (16d)$$

From eqs. (7),(8) and (16), one can obtain the following relations:

$$N_r = A_{11} \left( \frac{du}{dr} + \nu \frac{1}{r} u \right) + (B_{11} - \alpha E_{11}) \left( \frac{d\phi_1}{dr} + \nu \frac{1}{r} \phi_1 \right) - \alpha E_{11} \left( \frac{d\phi_2}{dr} + \nu \frac{1}{r} \phi_2 \right) \quad (17a)$$

$$N_\theta = A_{11} \left( \nu \frac{du}{dr} + \frac{1}{r} u \right) + (B_{11} - \alpha E_{11}) \left( \nu \frac{d\phi_1}{dr} + \frac{1}{r} \phi_1 \right) - \alpha E_{11} \left( \nu \frac{d\phi_2}{dr} + \frac{1}{r} \phi_2 \right) \quad (17b)$$

$$M_r = B_{11} \left( \frac{du}{dr} + \nu \frac{1}{r} u \right) + (D_{11} - \alpha F_{11}) \left( \frac{d\phi_1}{dr} + \nu \frac{1}{r} \phi_1 \right) - \alpha F_{11} \left( \frac{d\phi_2}{dr} + \nu \frac{1}{r} \phi_2 \right) \quad (17c)$$

$$M_\theta = B_{11} \left( \nu \frac{du}{dr} + \frac{1}{r} u \right) + (D_{11} - \alpha F_{11}) \left( \nu \frac{d\phi_1}{dr} + \frac{1}{r} \phi_1 \right) - \alpha F_{11} \left( \nu \frac{d\phi_2}{dr} + \frac{1}{r} \phi_2 \right) \quad (17d)$$

$$P_r = E_{11} \left( \frac{du}{dr} + \nu \frac{1}{r} u \right) + (F_{11} - \alpha H_{11}) \left( \frac{d\phi_1}{dr} + \nu \frac{1}{r} \phi_1 \right) - \alpha H_{11} \left( \frac{d\phi_2}{dr} + \nu \frac{1}{r} \phi_2 \right) \quad (17e)$$

$$Q_r = A_{44} \left( \phi_1 + \frac{dw}{dr} \right) - 3\alpha D_{44} (\phi_1 + \phi_2) \quad (17f)$$

$$R_r = D_{44} \left( \phi_1 + \frac{dw}{dr} \right) - 3\alpha F_{44} (\phi_1 + \phi_2) \quad (17h)$$

Where:

$A_{11}, B_{11}, D_{11}, E_{11}, F_{11}, H_{11}$  : are the circular disk stiffness coefficients

$$(A_{11}, B_{11}, D_{11}, E_{11}, H_{11}) = \int_{-h/2}^{h/2} \frac{E(z)}{(1-\nu^2)} (1, z, z^2, z^3, z^4, z^6) dz \quad (18a)$$

$$(A_{44}, D_{44}, F_{44}) = \int_{-h/2}^{h/2} \frac{E(z)}{2(1-\nu)} (1, z, z^2, z^4) dz \quad (18b)$$

From equation (12), the equilibrium equations are  $\delta u$ :

$$\frac{d}{dr}(rN_r) + N_\theta - \rho_1 r^2 \omega^2 = 0 \quad (19a)$$

$\delta \phi_1$ :

$$\frac{d}{dr}(rM_r) - \alpha \frac{d}{dr}(rP_r) - M_\theta + \alpha P_\theta - rQ_r + 3\alpha rR_r + (\rho_2 - \alpha\rho_3)r^2\omega^2 = 0 \quad (19b)$$

$\delta \phi_2$ :

$$\alpha \frac{d}{dr}(rP_r) - \alpha P_\theta - 3\alpha rR_r + \alpha\rho_3 r^2 \omega^2 = 0 \quad (19c)$$

$\delta w$ :

$$\frac{d}{dr}(rQ_r) + rq = 0 \quad (19d)$$

Equilibrium equations in terms of displacements:

$$(A_{11}r) \frac{d^2u}{dr^2} + A_{11} \frac{du}{dr} - A_{11} \frac{1}{r}u + (B_{11} - \alpha E_{11})r \frac{d^2\phi_1}{dr^2} + (B_{11} - \alpha E_{11}) \frac{d\phi_1}{dr} - (B_{11} - \alpha E_{11}) + \frac{1}{r}\phi_1 - \alpha E_{11}r \frac{d^2\phi_2}{dr^2} - \alpha E_{11} \frac{d\phi_2}{dr} + \alpha E_{11} \frac{1}{r}\phi_2 + \rho_1 r^2 \omega^2 = 0 \quad (20a)$$

$$(B_{11} - \alpha E_{11})r \frac{d^2u}{dr^2} + (B_{11} - \alpha E_{11}) \frac{du}{dr} - (B_{11} - \alpha E_{11}) \frac{1}{r}u + (D_{11} - 2\alpha F_{11} + \alpha^2 H_{11}) \frac{d\phi_1}{dr} - \left[ (D_{11} - 2\alpha F_{11} + \alpha^2 H_{11}) \frac{1}{r} + (A_{44} - 6\alpha D_{44} + 9\alpha^2 F_{44})r \right] \phi_1 - (\alpha F_{11} + \alpha^2 H_{11})r \frac{d^2\phi_2}{dr^2} - (\alpha F_{11} + \alpha^2 H_{11}) \frac{d\phi_2}{dr} + \left[ (\alpha F_{11} + \alpha^2 H_{11}) \frac{1}{r} + (3\alpha D_{44} + 9\alpha^2 F_{44})r \right] \phi_2 - (A_{44} - 3\alpha D_{44}) \frac{dw}{dr} + (\rho_2 - \rho_3)r^2\omega^2 = 0 \quad (20b)$$

$$E_{11}r \frac{d^2u}{dr^2} + E_{11} \frac{du}{dr} - E_{11} \frac{1}{r}u + (F_{11} + \alpha H_{11})r \frac{d^2\phi_1}{dr^2} + (F_{11} + \alpha H_{11}) \frac{d\phi_1}{dr} - \left[ (F_{11} + \alpha H_{11}) \frac{1}{r} + (3D_{44} - 9\alpha F_{44})r \right] \phi_1 - \alpha H_{11}r \frac{d^2\phi_1}{dr^2} - \alpha H_{11} \frac{d\phi_1}{dr} + \left( \alpha H_{11} \frac{1}{r} + 9\alpha E_{44}r \right) \phi_2 - 3D_{44}r \frac{dw}{dr} + \alpha\rho_3 r^2 \omega^2 = 0 \quad (20c)$$

$$(A_{44} - 3\alpha D_{44})r \frac{d\phi_1}{dr} + (A_{44} - 3\alpha D_{44})\phi_1 - (3\alpha D_{44})r \frac{d\phi_2}{dr} - (3\alpha D_{44})\phi_2 + A_{44}r \frac{d^2w}{dr^2} + rq = 0 \quad (20d)$$

Using the following dimensionless parameters for simplicity.

$$R = \frac{r}{a}, \quad W = \frac{w}{h}, \quad U = \frac{uh}{a^2}, \quad \varphi = \phi, \quad \delta = \frac{h}{a}, \quad \eta_1 = \frac{A_{11}h^6}{H_{11}}, \quad \eta_2 = \frac{B_{11}h^5}{H_{11}}, \quad \eta_3 = \frac{D_{11}h^4}{H_{11}}, \quad \eta_4 = \frac{E_{11}h^3}{H_{11}}, \quad \eta_5 = \frac{F_{11}h^2}{H_{11}}, \quad \eta_6 = \frac{A_{44}h^6}{H_{11}}, \quad \eta_7 = \frac{D_{44}h^4}{H_{11}}, \quad \eta_8 = \frac{F_{44}h^2}{H_{11}}$$

$Q$  : pressure parameter

$$Q = \frac{h^6 * q}{H_{11}}$$

$\zeta$  : bodyforec parameters

$$\zeta_1 = \frac{\rho_1 h^7 a \omega^2}{H_{11}}$$

$$\zeta_2 = \frac{(\rho_2 - \alpha \rho_3) h^8 \omega^2}{\alpha H_{11}}$$

$$\zeta_3 = \frac{4 \rho_3 h^4 \omega^2}{3 \alpha H_{11}}$$

the equations of motion in dimensionless form are:

$$\begin{aligned} & \eta_1 \delta R \frac{d^2 U}{dr^2} + \eta_1 \delta \frac{dU}{dr} - \eta_1 \delta \frac{1}{r} U + \left( \eta_2 - \frac{4}{3} \eta_4 \right) R \frac{d^2 \phi_1}{dR^2} + \left( \eta_2 - \frac{4}{3} \eta_4 \right) \frac{d\phi_1}{dR} - \\ & \left( \eta_2 - \frac{4}{3} \eta_4 \right) \frac{1}{R} \phi_1 - \frac{4}{3} \eta_4 R \frac{d^2 \phi_2}{dR^2} - \frac{4}{3} \eta_4 \frac{d\phi_2}{dR} + \frac{4}{3} \eta_4 \frac{1}{12} \phi_2 = -R^2 \zeta_1 \end{aligned} \quad (21a)$$

$$\begin{aligned} & \left( \eta_2 - \frac{4}{3} \eta_4 \right) \delta R \frac{d^2 U}{dR^2} + \left( \eta_2 - \frac{4}{3} \eta_4 \right) \delta \frac{dU}{dR} - \left( \eta_2 - \frac{4}{3} \eta_4 \right) \delta \frac{1}{R} U + \\ & \left[ \left( \eta_3 - \frac{8}{3} \eta_5 + \frac{16}{9} \right) R \frac{d^2 \phi_1}{dR^2} + \left( \eta_3 - \frac{8}{3} \eta_5 + \frac{16}{9} \right) \frac{d\phi_1}{dR} - \left[ \left( \eta_3 - \frac{8}{3} \eta_5 + \frac{16}{9} \right) \frac{1}{R} + \right. \right. \\ & \left. \left. \left( \eta_6 - 8\eta_7 + 16\eta_8 \right) \frac{1}{\delta^2} R \right] \phi_1 - \left( \frac{8}{3} \eta_5 - \frac{16}{9} \right) \right. \\ & \left. R \frac{d^2 \phi_2}{dR^2} - \left( \frac{8}{3} \eta_5 - \frac{16}{9} \right) \frac{d\phi_2}{dR} + \left[ \left( \frac{8}{3} \eta_5 - \frac{16}{9} \right) \frac{1}{R} + \left( 4\eta_7 + 16\eta_8 \right) \frac{1}{\delta^2} R \right] \phi_2 - \left( \eta_6 + 4\eta_7 \right) \right. \\ & \left. \frac{1}{\delta} R \frac{dw}{dR} = -\zeta_2 R^2 \end{aligned} \quad (21b)$$

$$\begin{aligned} & \eta_4 \delta R \frac{d^2 U}{dR^2} + \eta_4 \delta \frac{dU}{dR} - \eta_4 \delta \frac{1}{R} U \left( \eta_5 - \frac{4}{3} \right) R \frac{d^2 \phi_1}{dR^2} + \left( \eta_5 - \frac{4}{3} \right) \frac{d\phi_1}{dR} - \\ & \left[ \left( \eta_5 - \frac{4}{3} \right) \frac{1}{R} + \left( 3\eta_7 - 12\eta_8 \right) \frac{1}{\delta^2} R \right] \phi_1 - \frac{4}{3} R \frac{d^2 \phi_2}{dR^2} - \frac{4}{3} \frac{d\phi_2}{dR} \\ & + \left( \frac{4}{3} \frac{1}{R} + 12\eta_8 \frac{1}{\delta^2} R \right) \phi_2 - 3\eta_7 \frac{1}{\delta} R \frac{dw}{dR} = -\zeta_3 R^2 \end{aligned} \quad (21c)$$

$$\begin{aligned} & (\eta_6 - 4\eta_7) R \frac{d\phi_1}{dR} + (\eta_6 - 4\eta_7) \phi_1 - 4\eta_7 R \frac{d\phi_2}{dR} - \\ & 4\eta_7 \phi_2 + \eta_7 \delta R \frac{d^2 w}{dR^2} + \delta \eta_6 \frac{dw}{dR} = -QR \end{aligned} \quad (21d)$$

Boundary Conditions

Clamped circular plate

At  $R = 0$

$$U = 0, \phi_1 = 0, \phi_2 = 0, \frac{dw}{dR} = 0, Q_r = 0 \quad (22a)$$

At  $R = 1$

$$U = 0, \phi_1 = 0, \phi_2 = 0, W = 0 \quad (22b)$$

Roller support circular plate

At  $R = 0$

$$U = 0, \phi_1 = 0, \phi_2 = 0, \frac{dw}{dR} = 0 \quad (22c)$$

At  $R = a$

$$W = 0, N_r = 0, M_r = 0, P_r = 0 \quad (22d)$$

#### 4. IMPLEMENTATION OF GDQ METHOD

The generalized differential quadrature (DQ) method is adopted to solve the differential equations of the annular plate. The core of the DQ method is that the derivative of a function in a domain  $(0 \leq x \leq L)$  is approximated as a weighted linear summation of a function values at all discrete points in that domain. Thus, DQ method changes the governing differential equations into a set of corresponding simultaneous equations. To demonstrate the DQ method, consider the  $r$ th derivative of a function  $f(x)$  can be estimated as

$$\frac{\partial^r f(x)}{\partial x^r} \Big|_{x_i} = \sum_{k=1}^n D_{ik}^r f(x_k) \quad i=1,2,\dots,n$$

Where  $x_i$  are the discrete points in the variable domain,  $D_{ik}^r$ , and  $f(x_k)$  are the weighting coefficient and the function value at the discrete points. Thus, for the first-order derivatives, the weighting coefficients can be calculated as [25]

$$D_{ik}^{(1)} = \frac{\phi(x_i)}{(x_i - x_k) \phi(x_k)} \quad i, k = 1, 2, \dots, n, \quad i \neq k$$

$$\phi(x_i) = \prod_{i=1}^n (x_i - x_k)$$

Where  $i, k = 1, 2, \dots, n, i \neq k$

After that, the domain of the annular plate is divided into  $n$  grade points in  $r$  direction. Chebyshev polynomial is the best method to evaluate the grid points in the domain of the plate[25]:

$$r_i = 0.5 * \left[ 1 - \cos\left(\frac{i-1}{n-1} \pi\right) \right]_{i=1, 2, \dots, n}$$

The governing eqs. (21) can be discretized according to the GDQ method as follows:

$$\begin{aligned} & R_i \sum_{j=1}^n a_2(i, j) u_j + \eta_7 \delta \sum_{j=1}^n a_1(i, j) u_j - \eta_7 \delta \frac{1}{R_i} u_i + \left( \eta_2 - \frac{4}{3} \eta_4 \right) R_i \sum_{j=1}^n a_1(i, j) \phi_j^1 + \\ & \left( \eta_2 - \frac{4}{3} \eta_4 \right) \sum_{j=1}^n a_1(i, j) \phi_j^1 - \left( \eta_2 - \frac{4}{3} \eta_4 \right) \frac{1}{R_i} \phi_i^1 - \\ & \left( \frac{4}{3} \eta_4 \right) R_i \sum_{j=1}^n a_2(i, j) \phi_j^2 - \frac{4}{3} \eta_4 \sum_{j=1}^n a_2(i, j) \phi_j^2 + \frac{4}{3} \eta_4 \frac{1}{R_i} \phi_i^2 = -\zeta_1 R_i^2 \end{aligned} \tag{23a}$$

$$\begin{aligned} & \left( \eta_2 - \frac{4}{3} \eta_4 \right) \delta R_i \sum_{j=1}^n a_2(i, j) u_j + \left( \eta_2 - \frac{4}{3} \eta_4 \right) \delta \sum_{j=1}^n a_1(i, j) u_j - \left( \eta_2 - \frac{4}{3} \eta_4 \right) \delta \frac{1}{R_i} u_i + \\ & \left( \eta_3 - \frac{8}{3} \eta_5 + \frac{16}{9} \right) R_i \sum_{j=1}^n a_2(i, j) \phi_j^1 + \left( \eta_3 - \frac{8}{3} \eta_5 + \frac{16}{9} \right) \sum_{j=1}^n a_1(i, j) \phi_j^1 - \\ & \left[ \left( \eta_3 - \frac{8}{3} \eta_5 + \frac{16}{9} \right) \frac{1}{R_i} + (\eta_6 - 8\eta_7 + 16\eta_8) \frac{1}{\delta^2} R_i \right] \phi_j^1 - \left( \frac{4}{3} \eta_5 - \frac{16}{9} \right) R_i \sum_{j=1}^n a_2(i, j) \phi_j^2 \\ & - \left( \frac{4}{3} \eta_5 - \frac{16}{9} \right) \sum_{j=1}^n a_1(i, j) \phi_j^2 + \left[ \left( \frac{4}{3} \eta_5 - \frac{16}{9} \right) \frac{1}{R_i} + (4\eta_7 - 16\eta_8) \frac{1}{\delta^2} R_i \right] \\ & \phi_j^2 - (\eta_6 - 4\eta_7) \frac{1}{\delta} R_i \sum_{j=1}^n a_1(i, j) w_j = -\zeta_2 R_i^2 \end{aligned} \tag{23b}$$

$$\begin{aligned} & \eta_4 \delta R_i \sum_{j=1}^n a_2(i, j) u_j + \eta_4 \delta \sum_{j=1}^n a_1(i, j) u_j - \eta_4 \delta \frac{1}{R_i} u_i + \left( \eta_5 - \frac{4}{3} \right) R_i \sum_{j=1}^n a_2(i, j) \phi_j^1 + \left( \eta_5 - \frac{4}{3} \right) \\ & \sum_{j=1}^n a_1(i, j) \phi_j^1 - \left[ \left( \eta_5 - \frac{4}{3} \right) \frac{1}{R_i} + (3\eta_7 - 12\eta_8) \frac{1}{\delta^2} R_i \right] \phi_j^1 - \frac{4}{3} R_i \sum_{j=1}^n a_2(i, j) \phi_j^2 - \frac{4}{3} \sum_{j=1}^n a_1(i, j) \phi_j^2 \\ & + \left( \frac{4}{3} \frac{1}{R_i} + 12\eta_8 \frac{1}{\delta^2} R_i \right) \phi_j^2 - 3\eta_7 \frac{1}{\delta} R_i \sum_{j=1}^n a_1(i, j) w_j = -\zeta_3 R_i^2 \end{aligned} \tag{23c}$$

$$(\eta_6 - 4\eta_7) R_i \sum_{j=1}^n a_1(i, j) \phi_j^1 + (\eta_6 - 4\eta_7) \phi_i^1 -$$

$$(4\eta_7) R_i \sum_{j=1}^n a_1(i, j) \phi_j^2 - 4\eta_7 \phi_i^2 + \eta_6 \delta R_i \sum_{j=1}^n a_2(i, j) w_j + \delta \eta_6 \sum_{j=1}^n a_1(i, j) w_j = -QR \tag{23d}$$

The boundry conditions can be discretized by the DQ on:

Clamped circular plate .

$$U_1 = \phi_1^1 = \phi_1^2 = 0, \sum_{j=1}^n a_1(i, j) w_j = 0 \text{ at } R=0 \tag{24a}$$

$$U_n = \phi_n^1 = \phi_n^2 = W_n = 0 \text{ at } R=1 \tag{24b}$$

Roller support circular plate .

$$U_1 = \phi_1^1 = \phi_1^2 = 0, \sum_{j=1}^n a_1(i, j) w_j = 0 \text{ at } R=0 \tag{25a}$$

$$W_n = 0 \text{ at } R=1 \tag{25b}$$

$$\begin{aligned} & \eta_1 \delta \sum_{j=1}^n a_1(n, j) u_j + (v\eta_1 \delta) U_n + \left( \eta_2 - \frac{4}{3} \eta_4 \right) \sum_{j=1}^n a_1(n, j) \phi_j^1 + \\ & \left( v\eta_2 - \frac{4}{3} v\eta_4 \right) \phi_n^1 - \frac{4}{3} \eta_4 \\ & \sum_{j=1}^n a_1(n, j) \phi_j^2 - \frac{4}{3} v\eta_4 \phi_n^2 = 0 \dots \dots \dots \text{at } R=1 \end{aligned} \tag{25c}$$

$$\begin{aligned} & \eta_2 \delta \sum_{j=1}^n a_1(n, j) u_j + (v\eta_2 \delta) U_n + \left( \eta_3 - \frac{4}{3} \eta_5 \right) \sum_{j=1}^n a_1(n, j) \phi_j^1 + \\ & \left( v\eta_3 - \frac{4}{3} v\eta_5 \right) \phi_n^1 - \frac{4}{3} \eta_5 \\ & \sum_{j=1}^n a_1(n, j) \phi_j^2 - \frac{4}{3} v\eta_5 \phi_n^2 = 0 \dots \dots \dots \text{at } R=1 \end{aligned} \tag{25d}$$

$$\eta_4 \delta \sum_{j=1}^n a_1(n, j) u_j + (v \eta_4 \delta) U_n + \left( \eta_5 - \frac{4}{3} \right) \sum_{j=1}^n a_1(n, j) \phi_j^1 + \left( v \eta_5 - \frac{4}{3} v \right) \phi_n^1 - \frac{4}{3} \sum_{j=1}^n a_1(n, j) \phi_j^2 - \frac{4}{3} v \phi_n^2 = 0 \dots \dots \dots \text{at } R=1$$

(25e)

The congregation of the governing equations and the related boundary conditions lead to a set of simultaneous linear algebraic equations which can be write in matrix form as:

$$\begin{bmatrix} k_{bb} & k_{bd} \\ k_{db} & k_{dd} \end{bmatrix} \begin{Bmatrix} (\Delta_b) \\ (\Delta_d) \end{Bmatrix} = \begin{Bmatrix} (0) \\ (q) \end{Bmatrix} \quad (26)$$

Where  $k_{bb}$  and  $k_{bd}$  are the stiffness matrices of boundary of the boundary conditions and the size of it are  $8 \times 8$  and  $8 \times 4n-8$  respectively.

$k_{db}$  and  $k_{dd}$  are stiffness matrices of governing equations and have size of  $4n-8 \times 8$  and  $4n-8 \times 4n-8$  respectively.

The vector  $\Delta_b$  contains the displacements corresponds to the boundary points and is eliminated using the static condensation technique. The stiffness matrix eq.26 can be reduced into the form of

$$\left[ -k_{db} k_{bb}^{-1} k_{bd} + k_{dd} \right] \{ \Delta_d \} = \{ q \} \quad (27)$$

From the above equation the vector of domain displacements  $\Delta_d$  can be evaluated.

**5. VALIDATION OF THE RESULTS**

In order to examine the accuracy and efficiency of the results of this paper a comparative study made with another study to implement this, an axisymmetric bending of a clamped and roller-support functionally graded circular plate under uniformly distributed load q. metallic volume fraction power law distribution through plate thickness and all material properties are getting from Reddy et al. [3] As shown in the table 1. The evaluation of the comparison. Between the presented numerical analysis and Reddy's exact result are illustrated in table 2 for dimensionless maximum deflection,

$$D_c = \frac{E_c h^3}{12(1-\nu^2)}$$

With  $\frac{64WD_c}{qa^4}$  Where h and a are the

thickness and radius of the circular plate. The conclusion from these comparisons, an excellent agreement between these results.

**Table 1.** Mechanical properties of ceramic and metal of circular FGM plate [ 3 ]

material	Young's modulus( Gpa)	Poison's ratio,v
ceramic	151	0.288
metal	70	0.288

**Table 2.** Comparisons of the result got in the present paper to the result got by Reddy et al.[3] for maximum dimantion less diflaction of FGM circular plate for different values of p.

Material constant P	Reddy [3]		Present	
	Clamped plate	Roller support plate	Clamped plate	Roller support plate
0	2.979	10.822	2.979	10.822
2	1.623	5.925	1.608	5.921
4	1.473	5.414	1.467	5.410
6	1.404	5.155	1.399	5.150
8	1.362	4.993	1.357	4.989
10	1.333	4.882	1.329	4.880
15	1.289	4.714	1.287	4.713
20	1.265	4.619	1.264	4.613
25	1.250	4.559	1.248	4.558
30	1.239	4.517	1.238	4.5165
35	1.231	4.486	1.230	4.486
40	1.225	4.463	1.229	4.462
50	1.216	4.429	1.216	4.429
100	1.199	4.359	1.1987	4.359

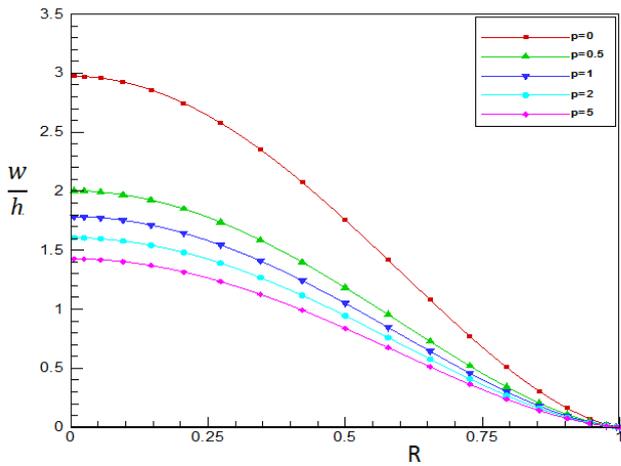
**6. RESULTS AND DISCUSSION**

In order to demonstrate the bending and stress analysis of FG circular plate numerically by unconstrained third order shear deformation theory via a generalized differential quadrature method, two cases are studied in this study, clamped circular plate and roller support circular disk.

**6.1 Clamped Circular Plate**

In the below, the results are presented in dimensionless form.

Fig (2) shows the dimensionless deflection  $\left( \frac{w}{h} \right)$  along the dimensionless radius (R) under bending load with different values of material constant P. As it expected, the deflection of the metallic plate (p=0) higher than that of the FG plate (p>0), because of the FG plate more stiffer than that of the pure metal plate.



**Figure 2:** Bending distribution ( $w/h$ ) for clamped circular plate with ( $R$ ) for different value of  $p$ .

Fig (3) present the variety of the dimensionless shear stress

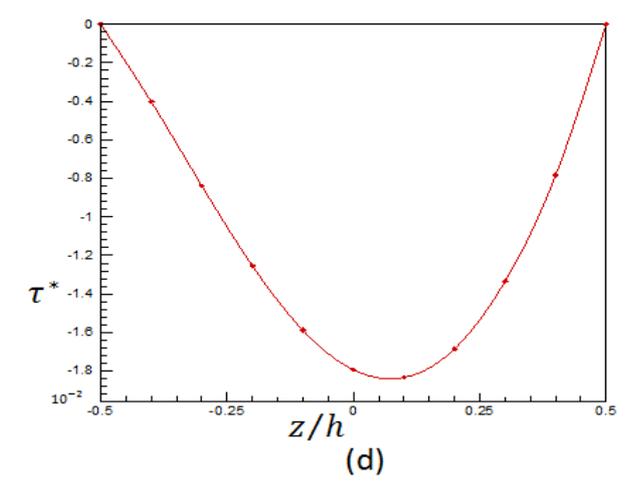
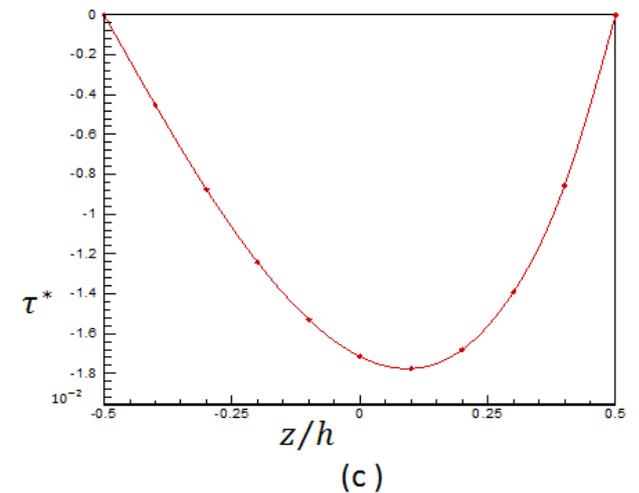
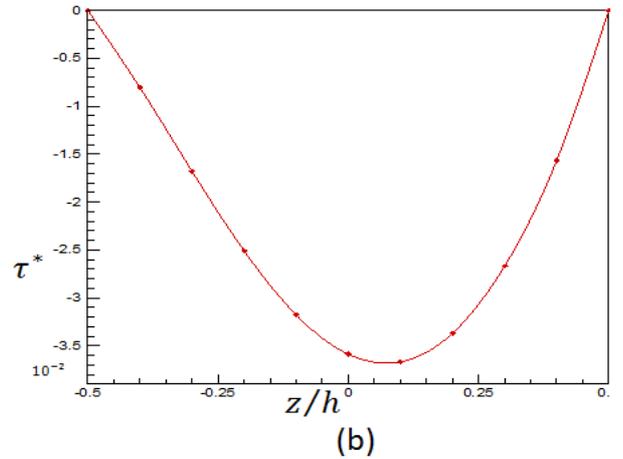
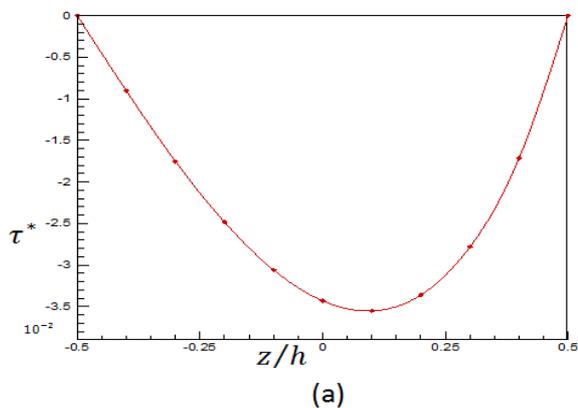
$$\left( \tau^* = \frac{\tau}{E_c} \right)$$

through the thickness of the plate with the dimensionless thickness coordinate variable  $\left( \frac{z}{h} \right)$  for different

values of material constant  $P$  and plate geometry  $\left( \frac{h}{a} \right)$ . It is seen that the values of shear stress decrease with increasing

in the thickness to the radius ratio  $(h/a)$  As well as , it is observable that , there is increasing in the shear stress with the increasing of the material constant  $P$  , moreover , it can be seen that the maximum value of shear stress does not occur in

the mid plane  $\left( \frac{z}{h} = 0 \right)$  ,the reason of this case belongs to the nature of non-homogenous of the mechanical properties of the FGMs.



**Figure 3:** Shear stress distribution through thickness for clamped circular plate (a)  $h/a = 0.1, p = 0.5$ , (b)  $h/a = 0.1, p = 2$  , (c)  $h/a = 0.2, p = 0.5$ , (d)  $h/a = 0.2, p = 2$

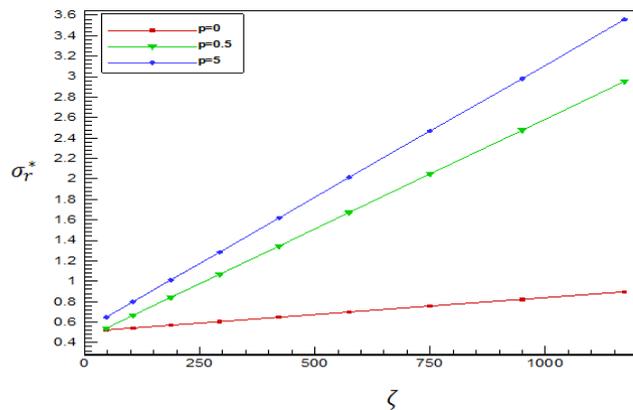
**6.2 Roller Support Circular Disk**

Fig (4) shows the radial stress parameter  $\left(\sigma_{r=}^* \frac{\sigma_r}{E_c}\right)$  with the body force parameter  $(\zeta)$ , for different values of the material constant  $p$ . It is clear that the radial stress parameter varies linearly with the body force parameter, moreover the radial stress in the FG disk is higher than that of a pure metallic disk ( $P=0$ ).

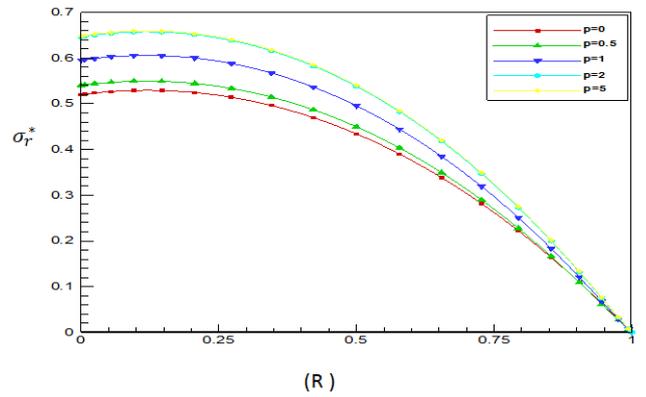
Fig (5) illustrates the variation of the radial stress parameter with  $(R)$  At  $\left(\frac{z}{h} = 0.25\right)$  and  $\left(\frac{z}{h} = -0.25\right)$ . It is clear that under uniform distributed load and body force, the radial stresses for the FG plate are higher than those of the pure metal plate ( $P=0$ ), because the FG plate has a higher density.

Fig (6) presents the dimensionless deflection  $\left(\frac{w}{h}\right)$  with  $(R)$  of the disk under bending load with different values of material constant  $p$ . It is clear that the deflection of the pure metal disk ( $p=0$ ) is higher than that of the FG plate, because of the rigidity of the FG plate.

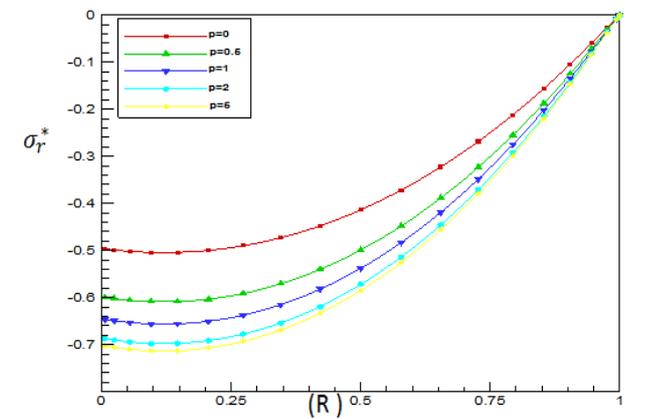
Fig (7) shows the distribution of shear stress parameter  $(\tau^*)$  through the thickness of the plate for different values of  $\frac{h}{a}$  and material constant  $P$ . It is clear that the behaviors of the shear stress through the thickness of the plate in this case of roller support condition are similar to the case of the clamped boundary condition.



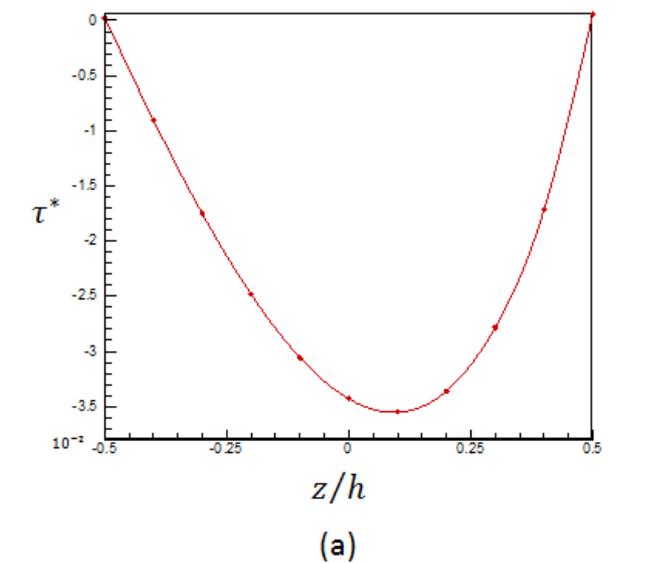
**Figure 4:** Radial stress  $\sigma_r^*$  distribution body force  $(\zeta)$  for roller support circular disk for different values of  $p$ .

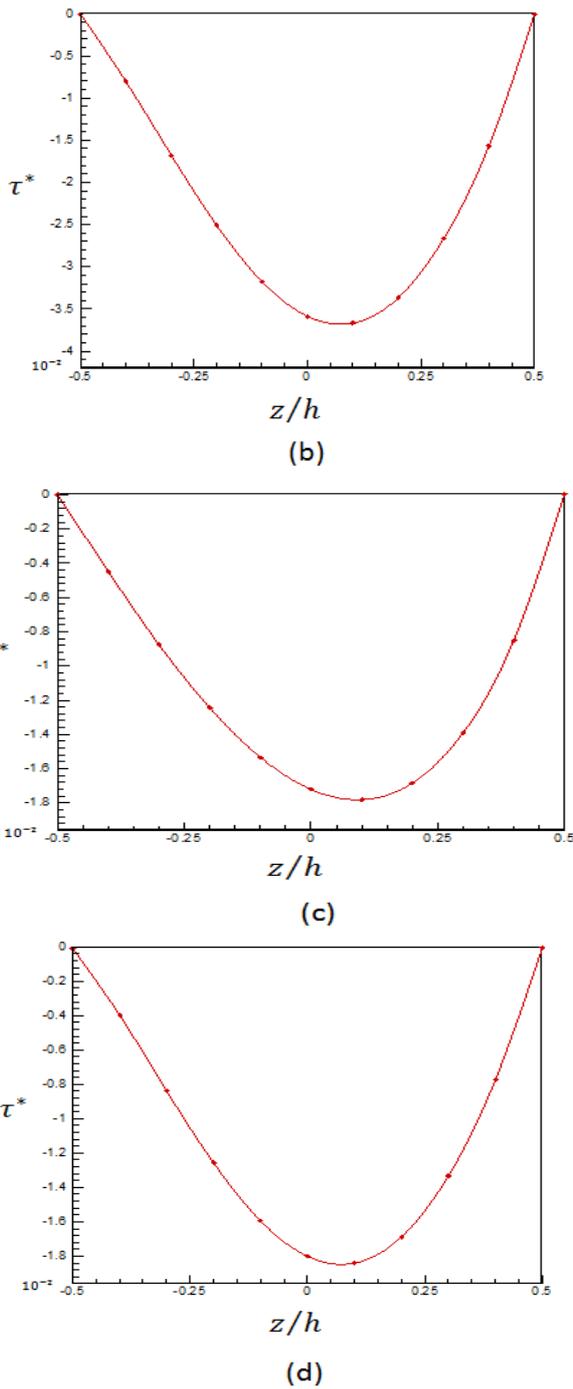


**Figure 5:** Radial stress  $\sigma_r^*$  configuration with  $(R)$  at  $\frac{z}{h} = 0.25$  for roller support circular disk for different values of  $p$ .

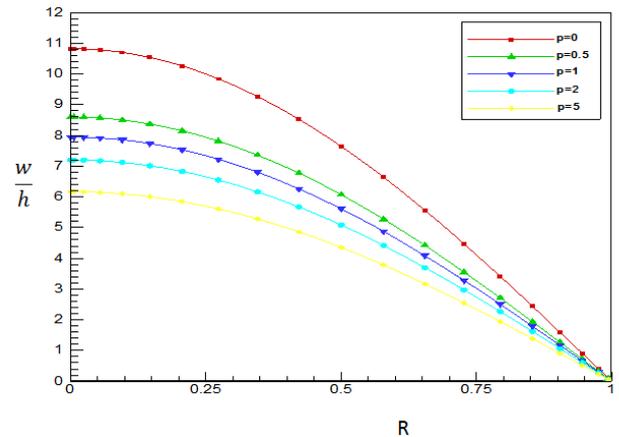


**Figure 6:** Radial stress  $\sigma_r^*$  configuration with  $(R)$  at  $\frac{z}{h} = -0.25$  for roller support circular disk for different values of  $p$ .





**Figure 7:** Shear stress distribution through thickness for roller support circular plate (a)  $h/a = 0.1, p = 0.5$ , (b)  $h/a = 0.1, p = 2$ , (c)  $h/a = 0.2, p = 0.5$ , (d)  $h/a = 0.2, p = 2$



**Figure 8:** Bending distribution ( $\frac{w}{h}$ ) for roller support circular plate with (R) for different value of p.

### CONCLUSIONS

An axisymmetric bending and stress analysis of functionally graded circular plate under uniform body force and uniform distributed load by unconstrained third order shear deformation theory via generalized differential quadrature method (DQM) the numerical solution of the unconstrained third order shear deformation theory can be applied to different case, of boundary condition, as well as, it can be applied to different loading condition, in contrast to the analytical solution limited to bending load.

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## BIOGRAPHIES



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